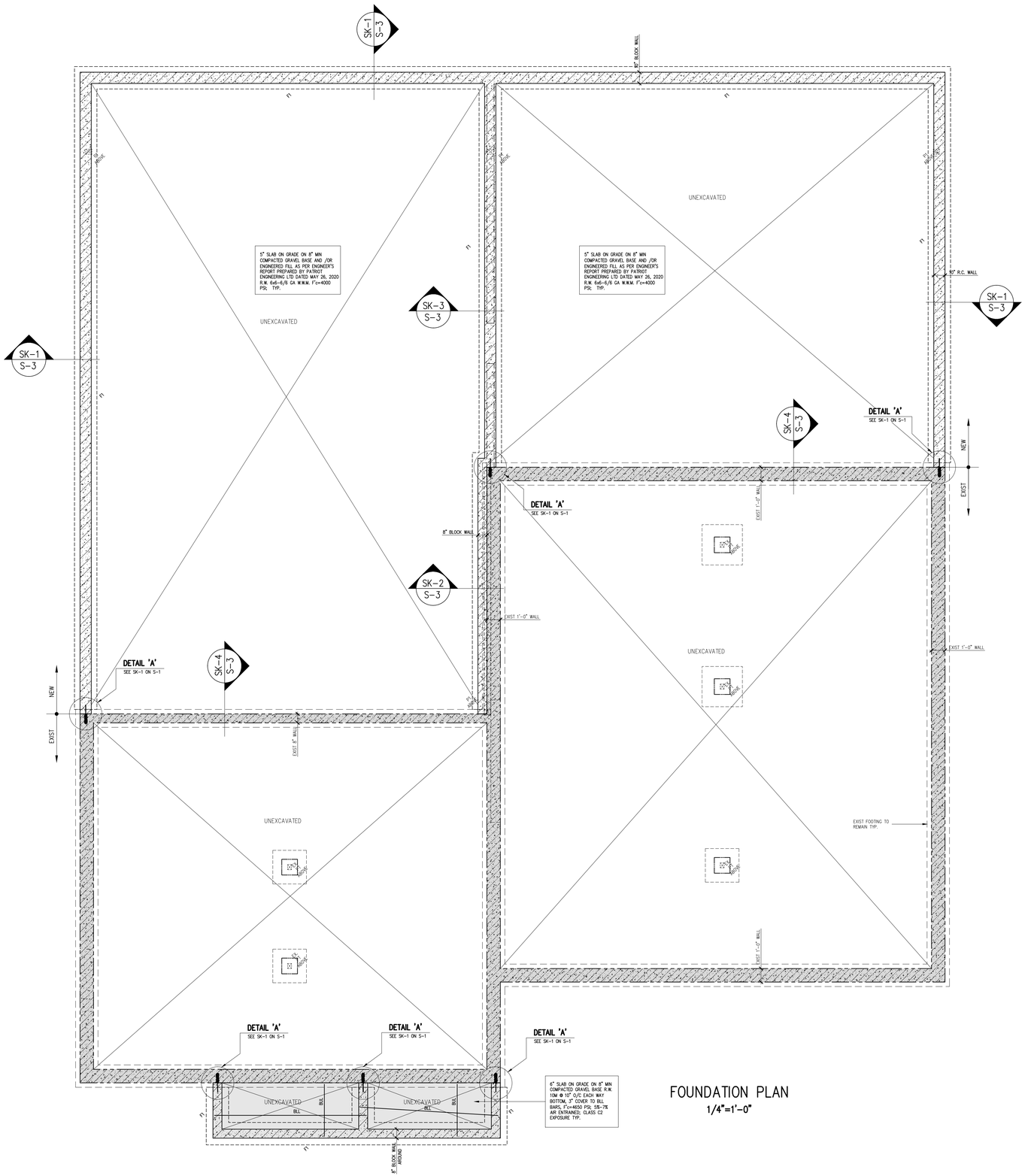


FOUNDATION PLAN
1/4"=1'-0"

ALL STRUCTURAL STEEL, CONNECTIONS AND CONNECTORS EXPOSED TO THE EXTERIOR ELEMENTS SHALL BE HOT DIP GALVANIZED

TYP. DEMOLITION/SHORING NOTE

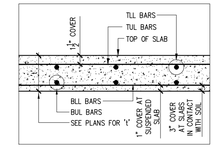
IT IS CONTRACTORS RESPONSIBILITY TO SHORE THE EXISTING STRUCTURE PRIOR TO THE CONSTRUCTION OF THE NEW STRUCTURE. CONSULT STRUCTURAL ENGINEER FOR RE COMMENDATIONS (TYP. AT SIMILAR) AND PROVIDE SHORING SHOP DRAWINGS STAMPED BY P. ENG OF ONTARIO FOR REVIEW AND COMMENTS TYP.

ALL EXISTING FRAMING AND STRUCTURAL INFORMATION NOTED ON DRAWINGS IS BASED ON EXISTING AVAILABLE INFORMATION AND ASSUMED CONDITIONS. CONTRACTOR SHALL REVIEW THE STRUCTURAL AND ARCHITECTURAL PLANS AND VERIFY THE ASSUMED CONDITIONS WITH EXISTING SITE CONDITIONS PRIOR TO COMMENCEMENT OF CONSTRUCTION. ADVISE STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES BETWEEN CONDITIONS NOTED ON THE DRAWINGS AND ACTUAL SITE CONDITIONS TYP. ALL LINTELS AND BEAMS NOTED ARE ASSUMED EXISTING UNLESS SPECIFICALLY NOTED ON PLANS TYP.

1. CONSULT ARCHITECTURAL DRAWINGS FOR ALL WATER PROOFING AND FLASHING DETAILS REQUIRED TO PROTECT THE CONCRETE STRUCTURE FROM THE EXTERIOR ELEMENTS PRIOR TO COMMENCEMENT WITH CONSTRUCTION.

2. CONSULT ARCHITECTURAL DRAWINGS FOR ALL FINISHED R.C. SLAB AND R.C. WALL ELEVATIONS.

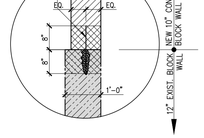
3. THE R.C. SLAB HAS BEEN DESIGNED TO SUPPORT THE LOADS OUTLINED ON THE PLANS. IT IS THE CONTRACTORS RESPONSIBILITY TO NOTIFY THE STRUCTURAL ENGINEER OF ANY DEVIATIONS FROM THE PROPOSED DESIGN LOADS, PROPOSED R.C. SLAB ELEVATIONS, PROPOSED BEARINGS AND PROPOSED REINFORCEMENT ARRANGEMENT.



TUL – TOP UPPER LAYER BARS (WHERE NOTED ON PLAN)
TLL – TOP LOWER LAYER BARS (WHERE NOTED ON PLAN)
BUL – BOTTOM UPPER LAYER BARS
BLL – BOTTOM LOWER LAYER BARS

TYP. R.C. SLAB REINFORCEMENT ARRANGEMENT

15M @ 24" O/C x 16" LONG GROUTED SOLID INTO NEW 10" BLOCK WALL WITH 2000 PSI GROUT TYP AND INTO EXIST. 10" BLOCK WALL WITH HILTI HIT HY 20 EPOXY INJECTION SYSTEM. INSTALL PRIOR TO ERECTING NEW 10" CONCRETE BLOCK WALL.



CONCEPTUAL PLAN DETAIL 'A'

DWG. NO. SK-1

FOUNDATION NOTES

1. THE CONTRACTOR TO CHECK WITH THE LOCAL MUNICIPALITY AND OTHERS FOR LOCATION OF CABLES, WATER PIPES, SEWERS, ETC. AND SHOULD BE SOLELY RESPONSIBLE FOR THEIR DAMAGE DURING THE CONSTRUCTION PERIOD.

2. FOOTINGS SHALL BEAR ON UNDISTURBED SOIL CAPABLE OF SUSTAINING A LOAD OF MIN. "ULS" BEARING CAPACITY OF 9000 PSF (EQUIVALENT TO "SLS" ALLOWABLE BEARING CAPACITY OF 6000 PSF).

3. ADVISE THIS OFFICE IF SOIL PRESSURE IS NOT AS PER NOTE ABOVE.

4. CONSULT SOIL ENGINEER'S REPORT PREPARED BY PATRIOT ENGINEERING LTD DATED MAY 26, 2020 FOR SPECIFICATIONS, RECOMMENDATIONS AND SOIL TEST RESULTS.

5. PROTECT SOIL FROM FREEZING ADJACENT TO AND BELOW ALL FOOTINGS.

6. PRIOR TO POURING OF CONCRETE FOOTINGS THE SOIL ENGINEER SHALL INSPECT SOIL CONDITIONS.

7. PRIOR TO POURING OF FOOTINGS ALL CENTRE LINES OF PIERS AND COLUMNS TO BE VERIFIED WITH ARCHITECTURAL AND STRUCTURAL DRAWINGS. ALL DISCREPANCIES TO BE REPORTED TO THIS OFFICE.

8. EXTERIOR FOOTINGS SHALL BE PLACED 4'-0" MINIMUM BELOW FINISHED GRADE.

9. THE SLOPE BETWEEN ADJACENT OR STEP FOOTINGS TO BE MAXIMUM OF 7 IN 10 AND STEPS SHALL BE 2'-0" MAXIMUM IN HEIGHT AND 4'-0" MINIMUM IN LENGTH.

10. CONCRETE SHALL BE F'c = 4000 PSI AT 28 DAYS AT INTERIOR OF THE STRUCTURE (TYP U/ND). EXTERIOR EXPOSED CONCRETE AT SLABS AND RETAINING WALLS SHALL BE 4650 PSI WITH 5%-7% AIR ENTRAINMENT, CLASS C2 EXPOSURE. TYP. U/ND.

11. ALL REINFORCING STEEL TO BE C.S.A. G30.1 DEFORMED BARS Fy = 60,000 PSI

12. CONSULT ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS OF PITS, SLAB ON GRADE DEPRESSIONS, ETC.

13. IT IS THE CONTRACTORS / PROPERTY OWNERS RESPONSIBILITY TO VERIFY PROPERTY SHORING REQUIREMENTS ALONG THE PROPRTY LINE WITH A SOILS ENGINEER PRIOR TO COMMENCEMENT OF EXCAVATION.

NO	REVISION DONE	DATE	BY
1	FOR PERMIT	AUGUST 10, 2020	G.SH.

GESCON – SHAFFIR
STRUCTURAL ENGINEERS INC.
TEL (416) 636-0700
FAX (416) 636-0469
e-mail:gescon@ican.net

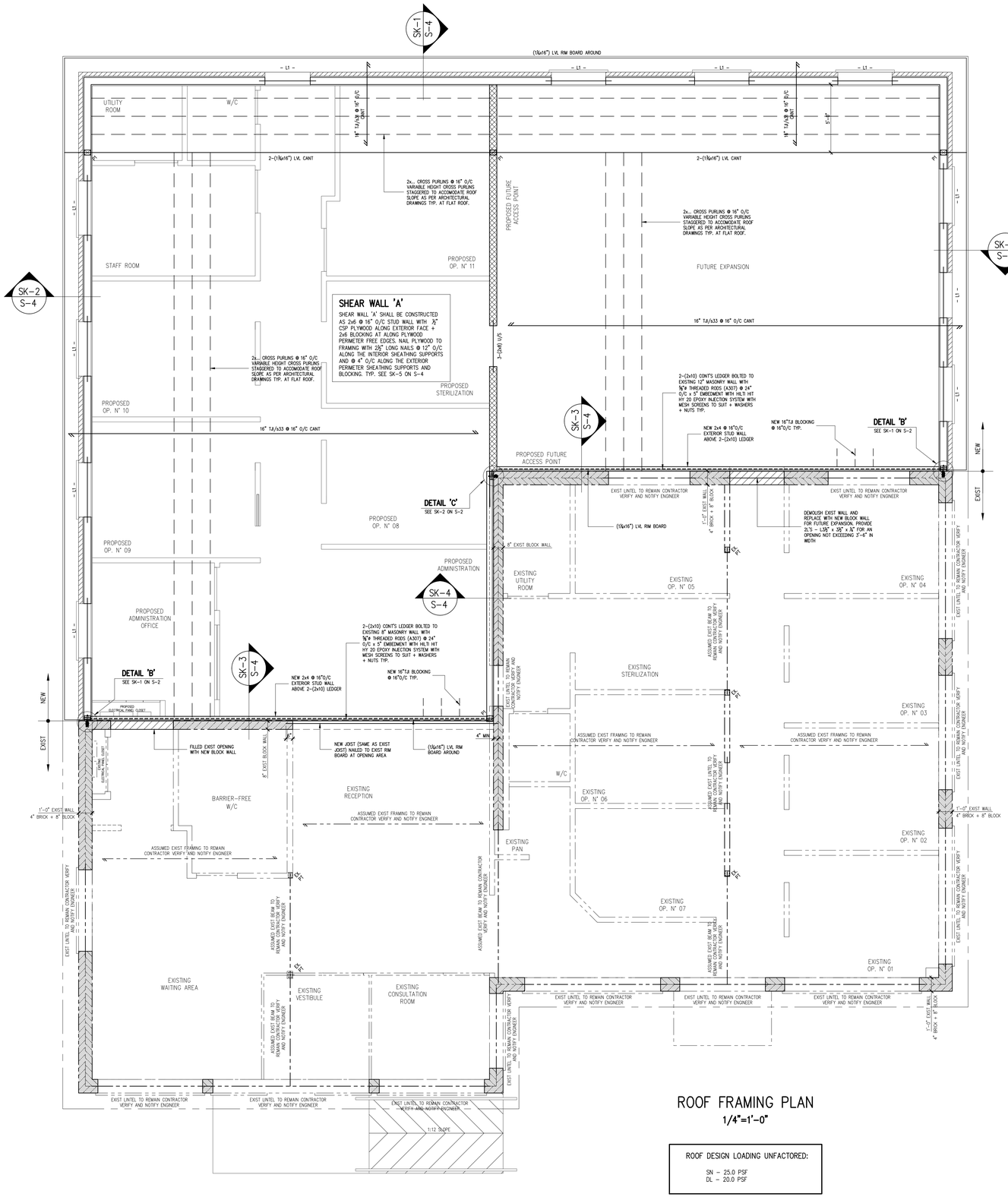
CLIENT
MICHAEL MANTZORIS ARCHITECT

PROJECT
510 TAUNTON ROAD EAST
OSHAWA ONTARIO

SHEET TITLE
FOUNDATION PLAN

LICENSED PROFESSIONAL ENGINEER – PROVINCE OF ONTARIO
AUGUST 10, 2020
Gideony A. Shaffir

DESIGNED	G.SH
DRAWN	X.T.
CHECKED	G.SH
SCALE	1/4" : 1'-0"
DATE	AUGUST 10, 2020
PROJECT NO.	0000
DWG NO.	S-1



ROOF FRAMING PLAN
1/4"=1'-0"

ROOF DESIGN LOADING UNFACTORED:

SN – 25.0 PSF
DL – 20.0 PSF

TYP. DEMOLITION/SHORING NOTE

IT IS CONTRACTORS RESPONSIBILITY TO SHORE THE EXISTING STRUCTURE PRIOR TO THE CONSTRUCTION OF THE NEW STRUCTURE. CONSULT STRUCTURAL ENGINEER FOR RE COMMENDATIONS (TYP. AT SIMILAR) AND PROVIDE SHORING SHOP DRAWINGS STAMPED BY P. ENG OF ONTARIO FOR REVIEW AND COMMENTS TYP.

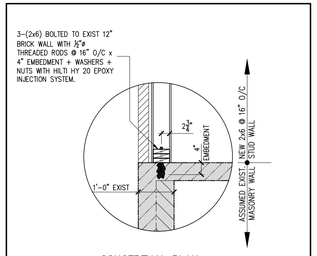
ALL EXISTING FRAMING AND STRUCTURAL INFORMATION NOTED ON DRAWINGS IS BASED ON EXISTING AVAILABLE INFORMATION AND ASSUMED CONDITIONS. CONTRACTOR SHALL REVIEW THE STRUCTURAL AND ARCHITECTURAL PLANS AND VERIFY THE ASSUMED CONDITIONS WITH EXISTING SITE CONDITIONS PRIOR TO COMMENCEMENT OF CONSTRUCTION. ADVISE STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES BETWEEN CONDITIONS NOTED ON THE DRAWINGS AND ACTUAL SITE CONDITIONS TYP. ALL LINTELS AND BEAMS NOTED ARE ASSUMED EXISTING UNLESS SPECIFICALLY NOTED ON PLANS TYP.

ALL STRUCTURAL STEEL, CONNECTIONS AND CONNECTORS EXPOSED TO THE EXTERIOR ELEMENTS SHALL BE HOT DIP GALVANIZED

ROOF VENTING NOTE :

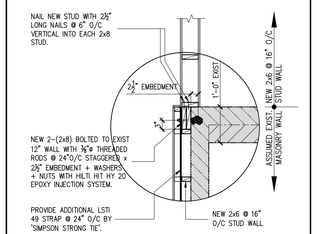
ALL ROOF VENTING DETAILS AND SPECIFICATIONS SHALL BE IN ACCORDANCE WITH PART 9 OF OBC AND ARCHITECTURAL DRAWINGS

ALL PURLINS NOTED SHALL BE STAGGERED (NOT CONTINUOUS)



CONCEPTUAL PLAN DETAIL 'B'

DWG. NO. SK-1



CONCEPTUAL PLAN DETAIL 'C'

DWG. NO. SK-2

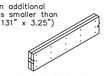
NOTE : TYP. AT ROOF FRAMING

* ROOF SHEETING SHALL BE ⅜" T&G SPF #1 AT FLAT ROOF AND ½" SPF AT SLOPED ROOF SUPPORTING SHINGLES AND ⅜" T&G SPF #1 AT SLOPED ROOF SUPPORTING SLATE,
* PROVIDE SKEWED HANGERS H1, H2, H3 AND H4 AS REQUIRED BY SITE CONDITIONS.
* JOISTS SHALL HAVE A MIN. BEARING OF 1 3/4" AT SUPPORTS TYP. U/ND
* WOOD/PSL/LVL BEAMS SHALL HAVE A MIN BEARING OF 3" AT NAILED POSTS P1 & P2 SUPPORT AND 4 ½"" BEARING AT BOLTED P3 POSTS SUPPORT. TYP. AT SIMILAR POST SIZES AND LOCATIONS U/ND.
* TOP OF ALL GIRDER TRUSSES NOTED '•' SHALL BE U/S ROOF JOISTS
* ALL 3-(2x10) & 2-(2x10) SHALL BE CONNECTED WITH LS70 EACH SIDE (2 TOTAL)
* TOP CHORD OF ALL GIRDER TRUSSES SUPPORTING CONVENTIONAL FRAMING SHALL BE 2x10 MIN.
* ALL RAFTERS SHALL BE CONNECTED TO GIRDER TRUSS TOP CHORD WITH LS70 SIMPSON STRONGTIE CONNECTOR
* ALL HIPS AND VALLEYS SHALL BE CONNECTED TO GIRDER TRUSSES & BEAMS WITH HANGERS AS NOTED OR HANGER H70 EACH SIDE (2 TOTAL)
* ALL 2x10 RAFTERS MAY BE SUBSTITUTED WITH JACK TRUSSES @ 16" O/C WITH 2x10 MIN. TOP CHORD CONT'S TYP.
* ALL LVL BEAMS SHALL BE 1.9E MIN. CAPACITY, Fb=4805 PSI MIN
* ALL PSL MEMBERS SHALL BE 2.0E MIN CAPACITY AT BEAMS, Fb=5360 PSI AND 1.8E MIN CAPACITY, Fb=4435 PSI AT COLUMNS AS PER TRUS-JOIST CATALOQUE.
* ALL LSL MEMBERS SHALL BE 1.7E MIN CAPACITY, Fb=4805 PSI AS PER TRUS-JOIST CATALOQUE.
* ALL RAFTERS CONNECTED TO TRUSS TOP CHORDS, HIPS, VALLEYS AND BEAMS SHALL HAVE H70 HANGERS AT ROOF FRAMING WHERE CEILING JOISTS ARE NOT BEARING ON EXTERIOR WALLS.

CONNECTION OF MULTIPLE PIECES OF TOP-LOADED BEAMS

1 3/4" Width Pieces:
- Minimum of 2 rows 16d (3 1/2") nails at 12" o.c.
- Minimum of 3 rows 16d (3 1/2") nails at 12" o.c. for 14", 16", 18", 18 3/4" and 19" beams
- Nailed connections require an additional row of nails when nail size is smaller than specified above (minimum 0.131" x 3.25")

3 1/2" Width Pieces:
- Minimum of 2 rows 1/2" bolts at 24" o.c., staggered



Multiple pieces of Trus Joist rectangular products can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7 inches. For side-loaded multiple member beams, additional nailing or bolting may be required See current Trus joist literature.

L6

INSTRUCTIONS TO TRUSS FABRICATOR :

THE DESIGN LAYOUT AND SUPPORT OF TRUSSES SHALL BE IN STRICT ACCORDANCE WITH THE ROOF FRAMING PLAN. NO ALTERNATE DESIGN SHALL BE APPROVED BY OUR OFFICE PRIOR TO CONSULTATIONS WITH 'GESCON-SHAFFIR STRUCTURAL ENGINEERS INC.'

MIN OBC NAILING REQUIREMENTS :

IT IS CONTRACTORS RESPONSIBILITY TO ENSURE COMPLIANCE WITH MIN. NAILING REQUIREMENTS AS PER OBC SECTIONS 9.23.3.4 & 9.23.3.5 UNLESS STRONGER CONNECTIONS ARE INDICATED ON PLANS AND SECTIONS.

NO	REVISION DONE	DATE	BY
1	FOR PERMIT	AUGUST DATE, 2020	G.SH.

GESCON – SHAFFIR
STRUCTURAL ENGINEERS INC.
TEL (416) 636-0700
FAX (416) 636-0469
e-mail: gescon@ica.net

CLIENT
MICHAEL MANTZORIS ARCHITECT

PROJECT
510 TAUNTON ROAD EAST
OSHAWA ONTARIO

SHEET TITLE
ROOF FRAMING PLAN

LICENSED PROFESSIONAL ENGINEER – PROVINCE OF ONTARIO – AUGUST 10, 2020 – Gideon A. Shaffir

DESIGNED	G.SH
DRAWN	X.T.
CHECKED	G.SH
SCALE	¼" : 1'-0"
DATE	AUGUST 10, 2020
PROJECT NO.	0000
DWG NO.	S-2

GENERAL STRUCTURAL NOTES

1. ALL CONSTRUCTION TO COMPLY WITH ONTARIO BUILDING CODE 2012 EDITION. DESIGN OF O.B.C. PART 4 AND PART 9 MEMBERS SHALL BE IN ACCORDANCE WITH THE FOLLOWING LOADING (TYP. U/N0):

GROUND FLOOR LOADING:
LL – 50.0 PSF
DL – 20.0 PSF

MAX LL DEFLECTION FOR ALL BEAM MEMBERS = L/360
MAX LL DEFLECTION FOR ALL JOIST MEMBERS = L/480

2. DRAWINGS SHALL NOT BE SCALED.

3. FOOTINGS SHALL BE POURED ON UNDISTURBED SOIL. EXTERNAL FOOTINGS SHALL BE ERECTED 4'-0" MINIMUM BELOW GRADE. MIN. 'SLS' DESIGN BEARING CAPACITY – 6000 PSF. SEE SOIL ENGINEER'S REPORT PREPARED BY PATRIOT ENGINEERING LTD DATED MAY 26, 2020 FOR SPECIFICATIONS, BACKFILL AND ENGINEERED FILL RECOMMENDATIONS AND SOIL TEST RESULTS. THE CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY THE DESIGN AND ACTUAL ON SITE BEARING CAPACITY WITH SOILS ENGINEER AND REPORT TO THE STRUCTURAL ENGINEER OF ANY DISCREPANCIES.

4. CONCRETE SHALL BE F'c = 4000 PSI TYP. U/N0. 5%–7% AIR ENTRAINED. CONSTRUCTION JOINTS SHALL BE LEFT ROUGH.

5. ALL CONCRETE CONSTRUCTION, WORKMANSHIP AND MATERIALS NOT NOTED IN PART 9 OF THE O.B.C. SHALL BE IN ACCORDANCE WITH CAN3-A23.3-04. ALL REINFORCEMENT SHALL BE DEFORMED BARS IN ACCORDANCE WITH CAN/CSA-G30.18-M92 WITH Fy=60 KSI. EXTEND CONTINUOS BARS INTO INTERSECTING MEMBERS FOR A DISTANCE OF 36 BAR DIAMETERS AND BENT IF REQUIRED. PROVIDE CONCRETE COVER FOR REINFORCEMENT AS REQUIRED BY O.B.C AND IN ACCORDANCE WITH CAN3-A23.3-04. DESIGN AND CONSTRUCTION OF PARKING STRUCTURES SHALL BE IN ACCORDANCE WITH CSA S413-07.

5A. ALL MASONRY CONSTRUCTION, WORKMANSHIP AND MATERIALS NOT NOTED IN PART 9 OF THE O.B.C. SHALL BE IN ACCORDANCE WITH CSA S304.1-04 DESIGN OF MASONRY STRUCTURE.

6. ALL STRUCTURAL STEEL CONSTRUCTION, WORKMANSHIP AND MATERIALS NOT NOTED IN PART 9 OF THE O.B.C. SHALL BE IN ACCORDANCE WITH CSA S16-09. STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH CSA-G40.21-04 GRADE 350W AND HSS SECTIONS SHALL BE G40.21-350W CLASS C. FABRICATION, CONNECTION DESIGN AND WELDING SHALL CONFORM TO CAN/CSA-S16-09. LATEST EDITION AND W59-03.B. ALL FABRICATION AND DESIGN OF STEEL DECK SHALL CONFORM TO CSA-S136-01 COLD FORMED STEEL STRUCTURAL MEMBERS AND TO CSSBI-B13-06 STANDARD FOR STEEL ROOF DECK.

ALL STEEL BEAMS SHALL BE ONLY TOP BEARING ON STEEL COLUMNS.

6A. DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS SHALL BE IN ACCORDANCE WITH CAN/CSA-S136-07 (USING APPENDIX B PROVISIONS).

7. MINIMUM BEARING OF STRUCTURAL MEMBERS ON MASONRY SHALL BE AS FOLLOWS:

CONCRETE AND STEEL BEAMS	7 1/2"
CONCRETE SLABS	4"
O.W.S.J.	4"
WOOD BEAMS AND JOISTS	4"

BEARING PLATES SHALL BEAR ON 3 COURSES OF 100% SOLID MASONRY WHICH SHALL EXTEND A MINIMUM OF 8" FROM EACH SIDE OF THE PLATE.

8. MASONRY:
MORTAR SHALL BE TYPE "S" OR BETTER WITH A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI. AT 28 DAYS. (TYP. U/N NOTED ON SECTIONS AND DETAILS) CONCRETE BLOCKS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OVER NET AREA IN ACCORDANCE WITH TABLE 9.20.2.7. AS PER PART 9 OF O.B.C. (TYP. U/N NOTED ON SECTIONS AND DETALS) ALL MASONRY CONSTRUCTION SHALL COMPLY WITH CAN/SCA-A371-04. CONCRETE BLOCK UNITS SHALL BE IN ACCORDANCE WITH CAN/CSA-A165.1-04.

9. REINFORCED MASONRY:
MORTAR SHALL BE TYPE "S" OR BETTER WITH A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS. CONCRETE BLOCKS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2950 PSI OVER NET AREA OF BLOCK. FILL CELLS CONTAINING REINFORCEMENT SOLID WITH GROUT. GROUT SHALL HAVE MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. LAP REINFORCING BARS 48 BAR DIAMETERS MINIMUM UNLESS OTHERWISE INDICATED ON PLANS. ALL REINFORCED MASONRY CONSTRUCTION SHALL COMPLY WITH CAN/SCA-A371-04.

9A. DESIGN AND CONSTRUCTION OF MASONRY CHIMNEYS AND FIREPLACES SHALL BE IN ACCORDANCE WITH CAN/CSA-A405-M87.

10. ALL DIMENSIONS AND EXISTING CONDITIONS SHALL BE VERIFIED BY THE GENERAL CONTRACTOR AT THE SITE. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE TO NOTIFY THIS OFFICE OF ANY DISCREPANCIES PRIOR TO COMMENCEMENT OF WORK.

11. FABRICATED ITEMS WHICH FABRICATION AND DESIGN IS NOT PRESCRIBED IN PART 9 OF THE O.B.C. SHALL BE PREENGINEERD AND DESIGNED IN ACCORDANCE WITH PART 4 OF THE O.B.C. SHOP DETAILS, DRAWINGS AND DIAGRAMS OF THESE ITEMS SHALL BE SUBMITTED TO THIS OFFICE FOR REVIEW PRIOR TO FABRICATION. THESE DRAWINGS SHALL BE SEALED BY A P. ENG OF ONTARIO RESPONSIBLE FOR THE DESIGN OF THESE ITEMS AND CLEARLY INDICATE THE METHOD OF CONNECTION OF THESE ITEMS TO THE STRUCTURE. THESE ITEMS SHALL INCLUDE STRUCTURAL STEEL, REINFORCING BARLISTS, CONNECTIONS BETWEEN WOOD MEMBERS AS PER HANGER SCHEDULE AND PRECAST, LIMESTONE AND MASONRY MEMBERS NOTED ON ARCHITECTURAL AND STRUCTURAL DRAWINGAS.

12. ALL FRAMING LUMBER SHALL BE SPF#1 UNLESS NOTED.

13. PLYWOOD SHALL BE 3/4" SPF#1 T & G AT FLOORS, 3/8" SPF#1 AT EXTERIOR STUD WALLS AND 1/2" SPF#1 AT SLOPED ROOF AND 5/8" SPF#1 T & G AT FLAT ROOF. PROVIDE EXTERIOR GRADE PLYWOOD WHERE REQUIRED BY O.B.C. CANADIAN SOFTWOOD PLYWOOD SHALL BE IN ACCORDANCE WITH CSA 0151-09.

14. ALL THE JOISTS AND BEAMS LOCATED AT THE SAME ELEVATION SHALL BE CONNECTED WITH JOIST HANGERS. ALL MEMBER CONNECTIONS SHALL MEET THE MINIMUM REQUIREMENTS AS OUTLINED IN PART 9 OF THE ONTARIO BUILDING CODE IN ACCORDANCE WITH TABLES 9.23.3.4 AND 9.23.3.5, UNLESS STRONGER CONNECTIONS ARE SPECIFIED.

15. ALL WOOD POSTS SHALL BE AS PER WOOD POST SCHEDULE. PROVIDE POST P1 AT ALL WOOD LINTEL BEARINGS UNLESS NOTED OTHERWISE ON PLANS. ALL WOOD POSTS SHALL BE CONT'S FROM FOOTINGS OR FOUNDATION WALLS TO U/S SUPPORTED BEAMS OR TRUSSES. PROVIDE SOLID BLOCKING AT DISCONTINUITIES SUCH AS FLOOR SPACES. (TYP. AT ALL WOOD POST LOCATIONS). PROVIDE 100 % SOLID BEARING U/S ALL POSTS AT BEARING. POSTS SHALL BEAR ON MINIMUM OF 3 COURSES OF SOLID MASONRY WHICH SHALL EXTEND A MINIMUM OF 8" FROM EACH SIDE OF THE PLATE OR SOLID CONCRETE.

16. HANGER SIZES SHALL BE AS PER HANGER SCHEDULE. THE HANGERS NOTED ABOVE ARE FOR INDICATION OF L/VL PLIES AND CONNECTION SHEAR FORCE CAPACITY ONLY. THE ACTUAL SHAPE OR ANGLE OF CONNECTION BETWEEN MEMBERS SHALL BE SURVEYED AT THE SITE BY THE HANGER DESIGNER. ALL HANGERS SHALL BE BY SIMPSON STRONG TIE OR EQUIVALENT. PROVIDE SHOP DRAWINGS FOR REVIEW AND COMMENTS SEALED BY P. ENG OF ONTARIO. DESIGN OF HANGERS SHALL BE IN ACCORDANCE WITH CAN3-O86-09.

17. ALL MICRO=LAM BEAMS AND "I" TYPE JOISTS HALL BE BY TRUS JOIST MACMILLAN OR EQUIVALENT. THE INSTALLATION OF THE MICRO=LAM BEAMS AND "I" JOISTS SHALL BE IN ACCORDANCE WITH THE MANUFACTURERS INSTALLATION GUIDE–LINES AND RECOMMENDATIONS.

18. THE LOAD BEARING STUD WALLS SHALL BE 2x6 @ 16" O/C SPF, #1 LUMBER, TYPICAL UNLESS NOTED. PROVIDE BRIDGING OR BLOCKING AT THE STUD WALLS TO GIVE 4'–0" MAXIMUM UNBRACED LENGTH. TYP. U/ND.

19. THE SPACING AND SIZES OF THE ROOF AND THE FLOOR JOISTS SHALL BE NOTED ON THE PLANS. PROVIDE FULL 2" SOLID BEARING AT THE SUPPORTS. PROVIDE 2x4 OR METAL CROSS BRIDGING AT 8'–0" O/C MAXIMUM.

20. THE DESIGN OF THE STRUCTURAL COMPOSITE LUMBER MEMBERS SHALL CONFORM TO CAN/CSA-O86-09 (LATEST EDDITION). ALL THE STRUCTURAL COMPOSITE LUMBER BEAMS SHALL BE OF MICRO=LAM LUMBER AS OUTLINED IN THE TRUS JOIST CANADA LTD. DESIGN CATALOGUE OR EQUIVALENT. THE INSTALLATION OF ALL THE STRUCTURAL COMPOSITE LUMBER BEAMS SHALL BE IN ACCORDANCE WITH THE MANUFACTURERS INSTALLATION GUIDE–LINES AND RECOMMENDATIONS

21. "I" TYPE JOISTS SHALL BE TJI JOISTS AS NOTED IN THE TRUS JOIST CANADA LTD. DESIGN CATALOGUE OR EQUIVALENT. SEE PLANS FOR THE LOCATION AND THE SPACING OF THE "I" JOISTS. THE INSTALLATION OF ALL "I" TYPE JOISTS SHAL BE IN ACCORDANCE WITH THE MANUFACTURERS INSTALLATION GUIDE–LINES AND RECOMMENDATIONS.

22. THE DESIGN AND ERECTION OF THE WOOD TRUSSES SHALL CONFORM TO THE CANADIAN STANDARD CSA–O86.1–09 AND THE ONTARIO BUILDING CODE.

23. THE TRUSS FABRICATOR SHALL SUBMIT SHOP DRAWINGS AND ERECTION DIAGRAMS TO THIS OFFICE FOR APPROVAL. THE DRAWINGS SHALL BE STAMPED BY A PROFESSIONAL EGINEER OF ONTARIO.

24. ALL TYPICAL AND NON–TYPICAL TRUSS BEARINGS SHALL BE CLEARLY INDICATED ON THE SHOP DRAWINGS. ALL REACTIONS OF THE TRUSSES AND THE TRUSS GIRDERS TO BE INDICATED ON THE SHOP DRAWINGS. LATERAL FORCES ON EXTERIOR BEARING WALLS ARE NOT ALLOWED.

25. THE ERECTION DIAGRAMS SHALL SPECIFY TEMPORARY AND PERMANENT BRACINGS, PROCEDURES AND METHODS REQUIRED BY THE FRAMING CONTRACTOR TO ERECT THE TRUSSES SUCCESFULLY.

26. CP1 SHALL BE 12"ø R.C. PIER TO U/S OF WOOD POSTS OR STEEL COLUMNS IN GARAGE R.W. 6x15M VERTICALS + 10M TIES @ 10" O/C. PROVIDE GALVANIZED COLUMN BASE CB6x6 BY MGA CONNECTORS AT WOOD POST ENSURE THAT U/S OF POST IS 6" ABOVE FLOOR EL.

27. ALL DIMENSIONS AND EXISTING CONDITIONS SHALL BE VERIFIED BY THE GENERAL CONTRACTOR AT THE SITE PRIOR TO CONSTRUCTION. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE TO NOTIFY THE ARCHITECT AND THE ENGINEER OF ANY DISCREPANCIES BETWEEN THE SITE CONDITIONS AND THE ASSUMED DESIGN CONDITIONS PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. IN ADDITION THE GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION, STRUCTURAL MEMBERS INCLUDING THE ERECTION OF STEEL BEAMS SUPPORTING EXISTING JOISTS. THE GENERAL CONTRACTOR SHALL SUBMIT SHORING DETAILS AND DRAWINGS STAMPED BY P. ENG. OF ONTARIO FOR REVIEW INDICATING THE SHORING PROCEDURE AND METHODS HE WILL EMPLOY TO SUPPORT EXISTING STRUCTURE. THE GENERAL CONTRACTOR SHALL EXERCISE EXTREME CAUTION AND CARE DURING THE DEMOLITION PROCESS OF THE EXISTING STRUCTURE AND MASONRY WALLS AND BE SOLELY RESPONSIBLE FOR THE SUPPOPT OF THE EXISTING STRUCTURE DURING THE DEMOLITION. THE GENERAL CONTRACTOR SHALL CALL THE STRUCTURAL ENGINEER FOR AN INSPECTION PRIOR TO CUTTING EXISTING MEMBERS AND REMOVING EXISTING WALLS.

28. ALL WOOD CONSTRUCTION SHALL CONFORM TO THE PART 9 OF O.B.C. AND CSA CAN–086.1–09. STRUCTURES UNDER PART 9 OF THE ONTARIO BUILDING CODE SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS UNDER PART 9 OF O.B.C. UNLESS NOTED OTHERWSE.

29. WOOD MEMBERS SHALL BE SPRUCE #1 GRADE AND/OR MICRO=LAM BRAND PARALLEL LAMINATED VENEER LUMBER WITH A FLEXURAL BENDING STRESS OF Fb = 19.3 MPA.

30. PROVIDE DOUBLE JOISTS UNDER ALL PARALLEL PARTITION WALLS.

31. LUMBER SHALL NOT BE TREATED WITH FIRE RETARDANTS WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.

32. MAXIMUM SPACING OF BRIDGING SHALL BE AT 1800 MM (6'–0") O/C AT JOISTS AND 1200 MM (4'–0") O/C AT LOAD BEARING STUD WALLS UNLESS NOTED OTHERWSE.

33. ALL CONNECTIONS BETWEEN WOOD MEMBERS SHALL BE "SIMPSON STRONG TIE CONNECTIONS".

34. GUARDS FOR HOUSING AND SMALL BUILDINGS SHALL COMPLY WITH SUPPLEMENTARY STANDARD SB–7, SEPTEMBER 14, 2012.

35. IT IS THE CONTRACTOR'S AND THE OWNERS RESPONSIBILITY TO SHORE EXCAVATION AND TO ENGAGE THE SERVCES OF A SOILS ENGINEER TO VERIFY THE STABILITY OF THE EXCAVATION. THIS IS REQUIRED WHEN THE DEPTH OF AN EXCAVATION EXCEEDS 4'–0" AND THE HORIZONTAL DISTANCE BETWEEN HIS EXCAVATION AND ADJACENT STRUCTURE AND /OR FOOTINGS AND/OR PROPERTY LINE IS LESS THAN 4'–0". IF SHORING IS REQUIRED PROVIDE SHOP DRAWINGS SEALED BY P. ENG. OF ONTARIO FOR REVIEW AND COMMENTS PRIOR TO COMMENCEMENT OF CONSTRUCTION.

FT.	SIZE	REINFORCEMENT	NOTES
F1	20"x 8" DP	2–15M CONT'S BOTTOM	TYPICAL AT 10" WALLS
F2	18"x 8" DP	2–15M CONT'S BOTTOM	TYPICAL AT 8" WALLS
CS1	12"ø PIER		

FOOTING SCHEDULE

LINTEL	STEEL 'L'	WOOD MEMBER	NOTES
L1	L 3 ½ x 3 ½ x ¼"	3 –(2x8)	
L2	L 4 x 3 ½ x ¼"	3 –(2x8)	
L3	L 5 x 3 ½ x 5/16" (LLV)	3 –(2x10)	
L4	L 6 x 3 ½ x 3/8" (LLV)	3 –(2x10)	

LINTEL SCHEDULE

COLUMN	SIZE	BASE PLATE	NOTES
C10	HSS 4x4x.313	BPL 7" x ⅜"x 7"	HOT DIP GALVANIZED

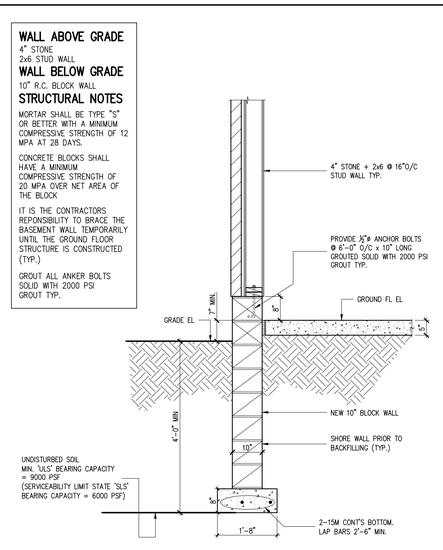
1. C10 SHALL HAVE CAP PLATE ₹ 4" X ⅜" x 10".

PROVIDE CONCRETE PAD – PAD1– 9 ½" WIDE x 16" LONG x 16" DEEP U/S ALL BASE PLATES AT BASE PLATE BEARING ON BLOCK WALLS. FILL BLOCKS SOLID U/S PAD1 AS PER NOTE 7 IN GENERAL STRUCTURAL NOTES.

STEEL COLUMN SCHEDULE

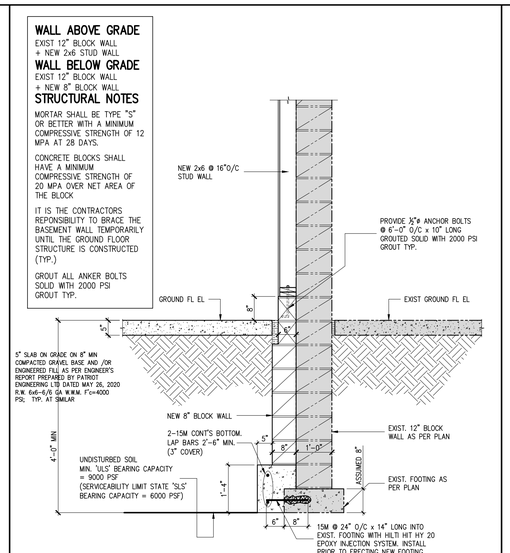
POST	SIZE	PLY NAILING/BOLTING	NOTES
P1	3–(2x6)	4" LONG ARDOX NAILS @ 10" O/C –2 ROWS	TYP. U/ND
P2	4–(2x6)	5" LONG ARDOX NAILS @ 10" O/C –2 ROWS	
P3	5 –(2x6) OR 5 ¼x7 PSL	3/8" ø THREADED RODS @ 10" O/C – 2 ROWS + WASHERS + NUTS AT 5–(2x6)	P.3 MAY BE SUBSTITUTED WITH P4. CONSULT BUILDER.
P4A	5 1/4 x 5 1/4 PSL	BY TRUS–JOIST OR EQUIVALENT	TYP. U/ND
P4	5 1/4 x 7 PSL		
P5	7 x 7 PSL		
P6	6x6	PREASSURE TREATED POST BEARING ON 100% SOLID BEARING 6" MIN. ABOVE FIN. GRADE ELEVATION.	POST CAPS ACB/AC66 & POST BASE CB 66 BY SIMPSON STRONG TIE OR EQUIVALENT (TYP.)
P6A	6x8		
P7	8x8		POST CAP LCE4 & POST BASE CB 88 BY SIMPSON STRONG TIE OR EQUIVALENT (TYP.)
P8	6–(2x6)	3/8" ø THREADED RODS @ 10" O/C –2 ROWS + WASHERS + NUTS	P8 MAY BE SUBSTITUTED WITH P9. CONSULT BUILDER.
P8A	7–(2x6)	3/8" ø THREADED RODS @ 10" O/C –2 ROWS + WASHERS + NUTS AT 7–(2x6)	TYP. U/ND
P8B	10–(2x6)	3/8" ø THREADED RODS @ 10" O/C –2 ROWS	
P9	5 1/4 x 9 1/2 PSL	BY TRUS–JOIST OR EQUIVALENT	
P11	3–(2x4)	4" LONG ARDOX NAILS @ 10" O/C –2 ROWS	
P12	4–(2x4)	5" LONG ARDOX NAILS @ 10" O/C –2 ROWS	

WOOD POST SCHEDULE



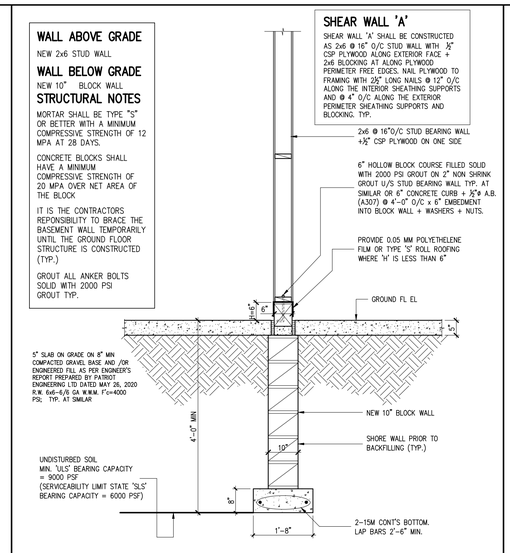
10" UNREINFORCED NEW CONCRETE BLOCK WALL AT EXTERIOR WALL

DWG. NO. SK–1



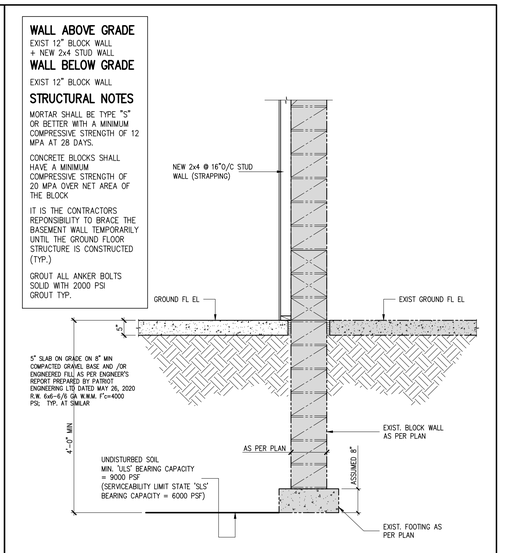
8" UNREINFORCED NEW CONCRETE BLOCK WALL AT INTERIOR EXIST 12" BLOCK WALL LOCATION

DWG. NO. SK–2



10" UNREINFORCED NEW CONCRETE BLOCK WALL AT IINTERIOR WALL

DWG. NO. SK–3



NEW 2x4 STUD WALL AT INTERIOR EXIST BLOCK WALL LOCATION

DWG. NO. SK–4

NO	REVISION DONE	DATE	BY
1	FOR PERMIT	AUGUST 10, 2020	G.SH.

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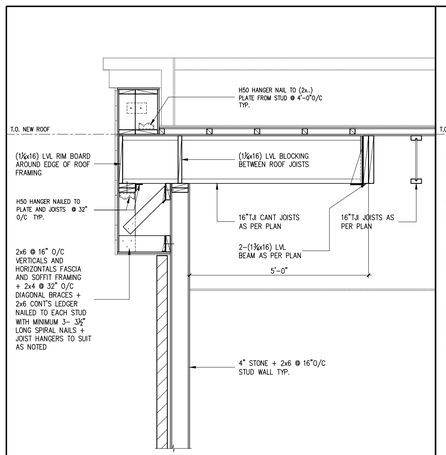
CLIENT
MICHAEL MANTZORIS ARCHITECT

PROJECT
510 TAUNTON ROAD EAST
OSHAWA ONTARIO

SHEET TITLE
GENERAL STRUCYURAL NOTES, TABLES AND SECTIONS

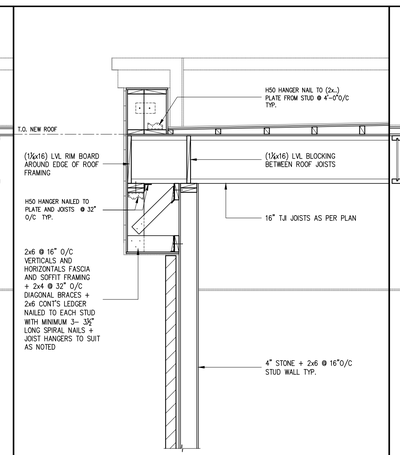
LICENSED PROFESSIONAL ENGINEER – PROVINCE OF ONTARIO
AUGUST 10, 2020
GideonyA. Shaffir

DESIGNED	G.SH
DRAWN	X.T.
CHECKED	G.SH
SCALE	¼" : 1'–0"
DATE	AUGUST 10, 2020
PROJECT NO.	0000
DWG NO.	S–3



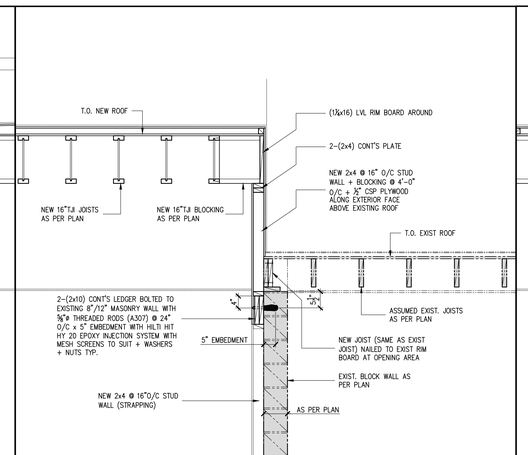
SECTION
1/2"=1'-0"

DWG. NO. SK-1



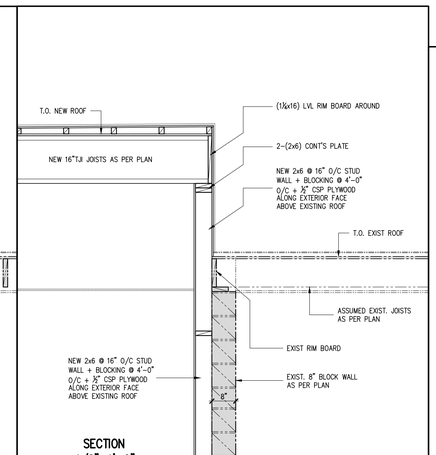
SECTION
1/2"=1'-0"

DWG. NO. SK-2



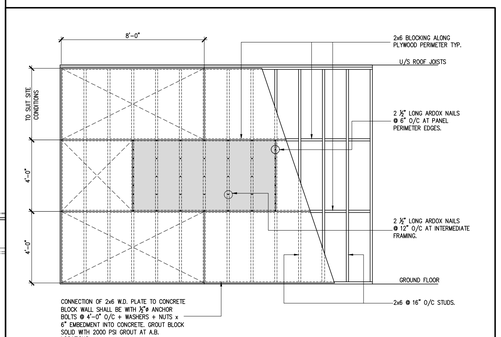
SECTION
1/2"=1'-0"

DWG. NO. SK-3



SECTION
1/2"=1'-0"

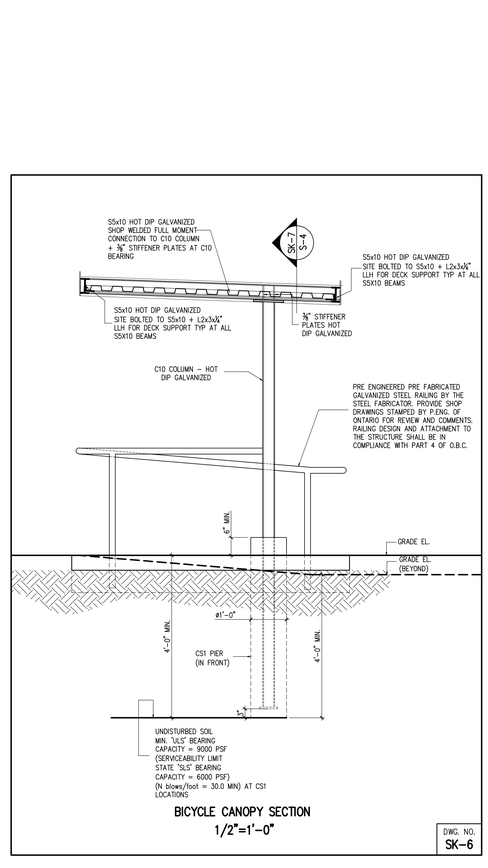
DWG. NO. SK-4



TYP. SHEAR WALL NAILING ELEVATION
AT SHEAR WALL 'A' (½" PLYWOOD)

1/4"=1'-0"

DWG. NO. SK-5

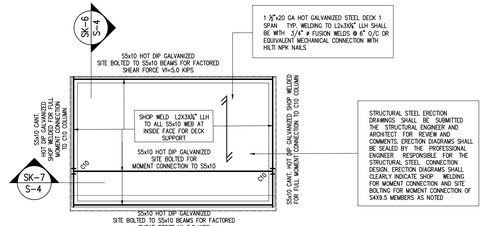


BICYCLE CANOPY SECTION
1/2"=1'-0"

DWG. NO. SK-6

BICYCLE CANOPY SECTION
1/2"=1'-0"

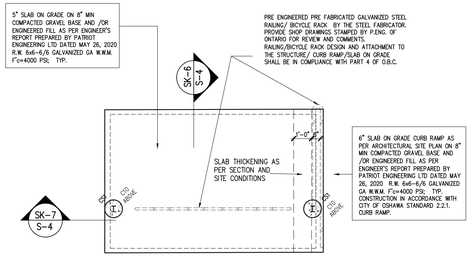
DWG. NO. SK-7



BICYCLE CANOPY ROOF FRAMING PLAN
1/4"=1'-0"

ROOF DESIGN LOADING UNFACTORED:

SN – 35.0 PSF
DL – 15.0 PSF
W – 22.0 PSF



BICYCLE CANOPY
FOUNDATION PLAN
1/4"=1'-0"

NO	REVISION DONE	DATE	BY
1	FOR PERMIT	AUGUST DATE, 2020	G.SH.

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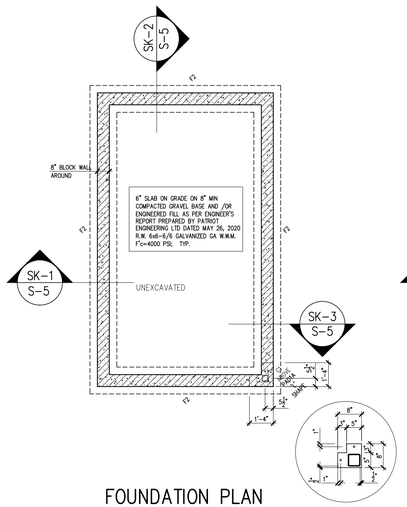
PROJECT
510 TAUNTON ROAD EAST
OSHAWA ONTARIO

SHEET TITLE
SECTIONS AND
BICYCLE CANOPY FRAMING PLANS

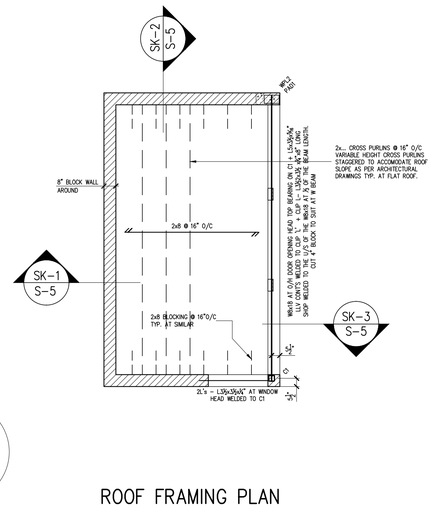
LICENSED PROFESSIONAL ENGINEER
AUGUST 10, 2020
Gideon A. Shaffir
PROVINCE OF ONTARIO

DESIGNED	G.SH
DRAWN	X.T.
CHECKED	G.SH
SCALE	¼" : 1'-0"
DATE	AUGUST 10, 2020
PROJECT NO.	0000
DWG NO.	S-4



FOUNDATION PLAN

1/4"=1'-0"



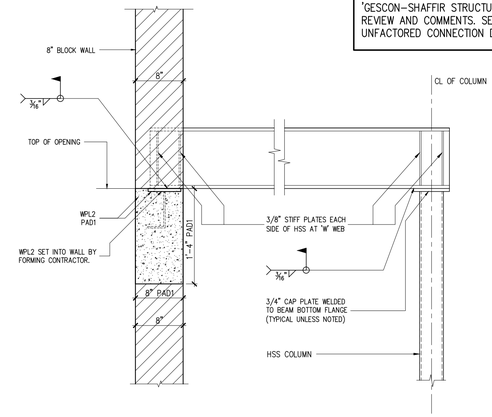
ROOF FRAMING PLAN

1/4"=1'-0"

ROOF DESIGN LOADING UNFACTORED:

SN – 25.0 PSF
DL – 20.0 PSF

ALL STRUCTURAL STEEL CONNECTIONS SHALL BE PRE ENGINEERED AND SHOP DRAWINGS STAMPED BY P.ENG. OF ONTARIO SHALL BE SUBMITTED TO GESCON-SHAFFIR STRUCTURAL ENGINEERS FOR REVIEW AND COMMENTS. SEE PLANS FOR UNFACTORED CONNECTION DESIGN LOADS NOTED.



BEAM BEARING ON STEEL COLUMN AND CONCRETE BLOCK WALL DETAIL (CONCEPTUAL)

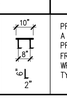
DWG. NO. SK-4

COLUMN	SIZE	BASE PLATE	NOTES
C1	HSS 4 x 4 x .25	'L' SHAPE PL 10" x ½" x 5" + 2 – ½"ø HILTI KWIK BOLTS x 4 ½" EMBEDMENT + WASHERS + NUTS	TYP. U/ND

1. C1 SHALL HAVE CAP PLATE ℄ 5" x ¼" x 8";

PAD1A – CONCRETE PAD 7 ½" WIDE x 16" LONG x 16" DEEP U/S C1 ;
FILL BLOCKS SOLID U/S PAD1 AND PAD2 AS PER NOTE 7 IN GENERAL STRUCTURAL NOTES.

STEEL COLUMN SCHEDULE

WALL PLATE	SIZE	ANCHORAGE		BEARING	NOTES
WPL2	5 ½" x ⅜" x 10"	2– ½"ø A.B. x 6" LONG		PROVIDE PAD1 U/S WPL2 A AS NOTED ON PLANS. PROVIDE SOLID BEARING FROM FOOTINGS TO U/S WPL AS NOTED ON PLANS TYP.	TYP. U/ND
					TYP. U/ND

WELD ALL BEAMS TO WALL PLATES.

PROVIDE CONCRETE PADS (F'c=3000 PSI) :

PAD1 – CONCRETE PAD 7 ½" WIDE x 16" LONG x 16" DEEP U/S WPL2

FILL BLOCKS SOLID FROM FLOOR EL. TO U/S PAD1 AND PAD2 WITH 2000 PSI GROUT MIN.

WALL PLATE SCHEDULE

FT.	SIZE	REINFORCEMENT	NOTES
F1	20"x 8" DP	2–15M CONT'S BOTTOM	TYPICAL AT 10" WALLS
F2	18"x 8" DP	2–15M CONT'S BOTTOM	TYPICAL AT 8" WALLS

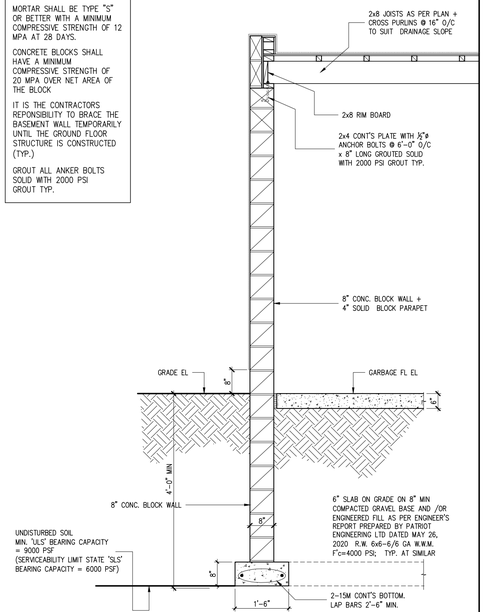
FOOTING SCHEDULE

WALL ABOVE GRADE
8" CONC. BLOCK WALL
WALL BELOW GRADE
8" CONC. BLOCK WALL
STRUCTURAL NOTES
MORTAR SHALL BE TYPE "S" OR BETTER WITH A MINIMUM COMPRESSIVE STRENGTH OF 12 MPA AT 28 DAYS.

CONCRETE BLOCKS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 20 MPA OVER NET AREA OF THE BLOCK

IT IS THE CONTRACTORS REPONSIBILITY TO BRACE THE BASEMENT WALL TEMPORARILY UNTIL THE GROUND FLOOR STRUCTURE IS CONSTRUCTED (TYP.)

GROUT ALL ANKER BOLTS SOLID WITH 2000 PSI GROUT TYP.



8" UNREINFORCED CONCRETE BLOCK WALL AT JOISTS BEARING ON WALL

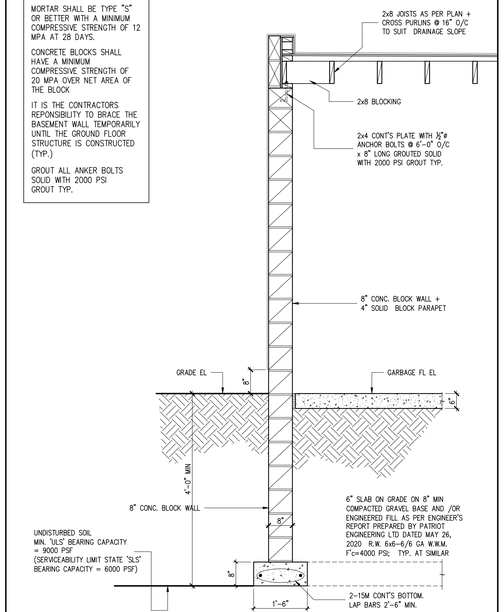
DWG. NO. SK-1

WALL ABOVE GRADE
8" CONC. BLOCK WALL
WALL BELOW GRADE
8" CONC. BLOCK WALL
STRUCTURAL NOTES
MORTAR SHALL BE TYPE "S" OR BETTER WITH A MINIMUM COMPRESSIVE STRENGTH OF 12 MPA AT 28 DAYS.

CONCRETE BLOCKS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 20 MPA OVER NET AREA OF THE BLOCK

IT IS THE CONTRACTORS REPONSIBILITY TO BRACE THE BASEMENT WALL TEMPORARILY UNTIL THE GROUND FLOOR STRUCTURE IS CONSTRUCTED (TYP.)

GROUT ALL ANKER BOLTS SOLID WITH 2000 PSI GROUT TYP.



8" UNREINFORCED CONCRETE BLOCK WALL AT JOISTS BEARING PARALLEL TO WALL

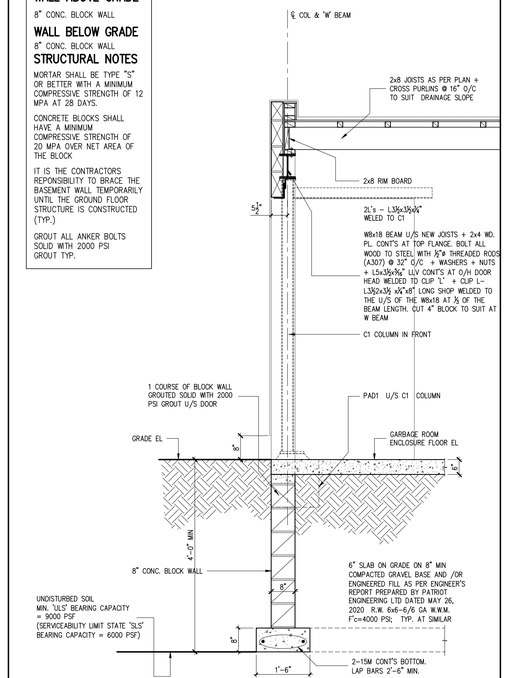
DWG. NO. SK-2

WALL ABOVE GRADE
8" CONC. BLOCK WALL
WALL BELOW GRADE
8" CONC. BLOCK WALL
STRUCTURAL NOTES
MORTAR SHALL BE TYPE "S" OR BETTER WITH A MINIMUM COMPRESSIVE STRENGTH OF 12 MPA AT 28 DAYS.

CONCRETE BLOCKS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 20 MPA OVER NET AREA OF THE BLOCK

IT IS THE CONTRACTORS REPONSIBILITY TO BRACE THE BASEMENT WALL TEMPORARILY UNTIL THE GROUND FLOOR STRUCTURE IS CONSTRUCTED (TYP.)

GROUT ALL ANKER BOLTS SOLID WITH 2000 PSI GROUT TYP.



SECTION AT GARBAGE ROOM ENCLOSURE DOOR OPENING

DWG. NO. SK-3

ALL STRUCTURAL STEEL, CONNECTIONS AND CONNECTORS EXPOSED TO THE EXTERIOR ELEMENTS SHALL BE HOT DIP GALVANIZED

NO	REVISION DONE	DATE	BY
1	FOR PERMIT	AUGUST DATE, 2020	G.SH.

GESCON – SHAFFIR
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FAX (416) 636-0469
e-mail: gescon@ica.net

CLIENT

MICHAEL MANTZORIS ARCHITECT

PROJECT

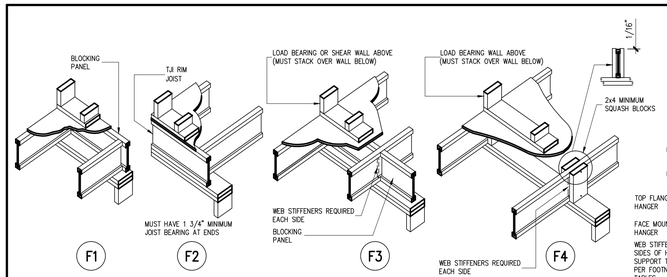
510 TAUNTON ROAD EAST
OSHAWA ONTARIO

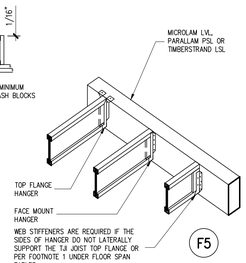
SHEET TITLE

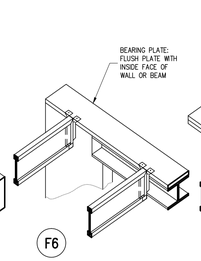
NEW GARBAGE ROOM ENCLOSURE PLANS

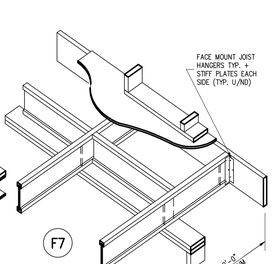
LICENSED PROFESSIONAL ENGINEER
AUGUST 10, 2020
Gideon A. Shaffir
PROVINCE OF ONTARIO

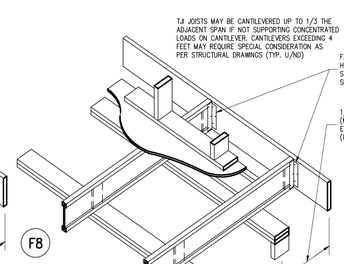
DESIGNED	G.SH
DRAWN	X.T.
CHECKED	G.SH
SCALE	¼" : 1'–0"
DATE	AUGUST 10, 2020
PROJECT NO. 0000	DWG NO. S-5

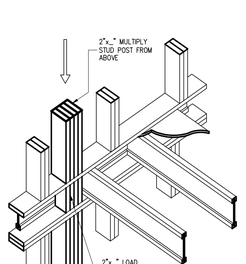


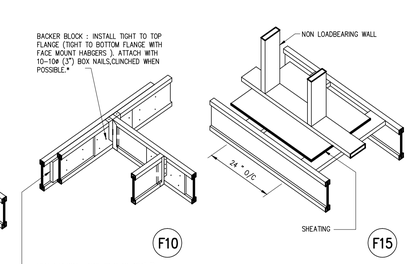






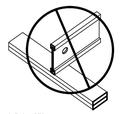






THESE CONDITIONS ARE NOT PERMITTED

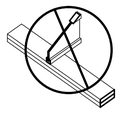
DO NOT CUT HOLES TOO CLOSE TO SUPPORTS.



REFER TO HOLE CHARTS TO 'TRUS JOIST MACMILLAN' CATALOG, PAGE 19.

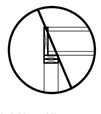
F11

DO NOT BEVEL CUT JOIST BEYOND INSIDE FACE OF WALL.



F12

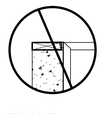
DO NOT USE SOLID-SAWN DIMENSION LUMBER FOR RIM BOARD OR BLOCKING.



DIMENTION LUMBER MAY SHRINK AFTER INSTALLATION.

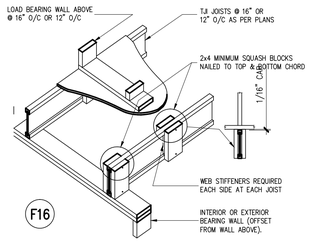
F13

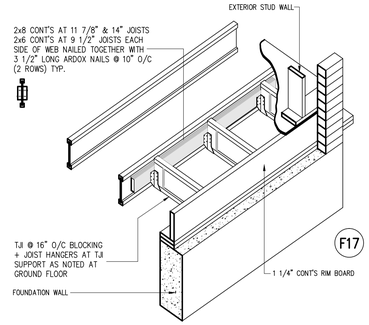
DO NOT INSTALL HANGER OVER-HANGING FACE OF PLATE OR BEAM.

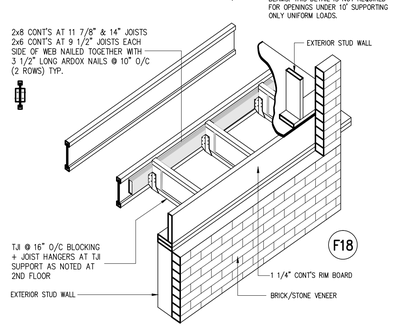


FLUSH BEARING PLATE WITH INSIDE FACE OF WALL OR BEAM.

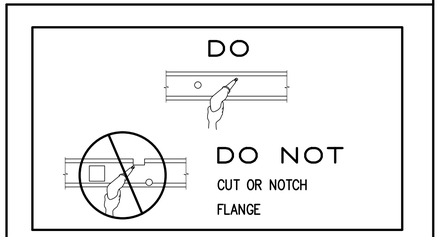
F14







FLOOR DETAILS



THIS DRAWING HAS BEEN PREPARED IN ACCORDANCE WITH CONSTRUCTION DETAILS OF TRUS-JOIST MACMILLAN. IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY COMPLIANCE OF THESE DETAILS WITH CONSTRUCTION DETAILS OF EQUIVALENT JOISTS. CONSULT EQUIVALENT JOIST FABRICATORS FOR CONSTRUCTION DETAILS AND RECOMMENDATIONS.

HOW TO USE THIS CHARTS

1. DETERMINE THE HOLE SHAPE GROUND SQUARE OR RECTANGULAR AND SELECT THE APPROPRIATE **CHART – A** OR **B.**
2. UNDER **HOLE SIZE,** LOCATE THE COLUMN WHICH MEETS OR EXCEEDS THE SIZE OF HOLE YOU REQUIRE.
3. USE THE FIRST TWO COLUMNS TO IDENTIFY THE TJI JOIST SERIES AND DEPTH BEING USED IN YOUR FLOOR OR ROOF SYSTEM.
4. SCAN RIGHT ACROSS THE ROW UNTIL YOU INTERSECT THE COLUMN WHICH CONTAINS THE HOLE SIZE SELECTED. THE VALUE SHOWN IS THE REQUIRED MINIMUM DISTANCE FROM EDGE OF THE HOLE TO THE INSIDE FACE OF THE NEAREST SUPPORT.

THESE CHARTS **DO NOT** APPLY TO THE VENTED 16" TJI/PRO 250 JOIST. CONTACT YOUR TRUS JOIST MAC MILLAN REPRESENTATIVE FOR ASSISTANCE.

CHART A – ROUND HOLES

MINIMUM DISTANCE FROM INSIDE FACE OF ANY SUPPORT TO NEAREST EDGE OF HOLE

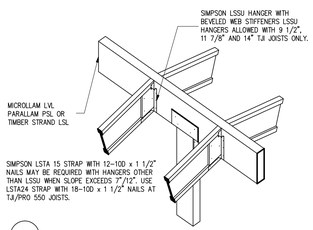
DEPTH	TJI/PRO™	ROUND HOLE SIZE												
		2"	3"	4"	5"	6"	6 1/4"	7"	8"	8 5/8"	9"	10"	10 3/4"	12"	12 3/4"
9 1/2"	150	1'–0"	1'–6"	3'–0"	4'–0"	7'–0"	7'–6"								
	250	1'–0"	2'–6"	4'–0"	5'–6"	7'–6"	8'–0"								
11 7/7"	150	1'–0"	1'–0"	1'–0"	1'–0"	3'–0"	3'–6"	5'–0"	7'–0"	8'–6"					
	250	1'–0"	1'–0"	2'–0"	3'–0"	4'–6"	5'–0"	6'–0"	8'–0"	9'–0"					
	350	1'–0"	2'–0"	3'–0"	4'–6"	5'–6"	6'–0"	7'–0"	9'–0"	10'–0"					
	550	1'–0"	1'–6"	3'–0"	4'–6"	6'–0"	6'–6"	7'–6"	9'–6"	10'–6"					
14"	250	1'–0"	1'–0"	1'–0"	1'–0"	1'–6"	2'–0"	3'–0"	5'–0"	6'–0"	6'–6"	8'–6"	10'–0"		
	350	1'–0"	1'–0"	1'–0"	1'–6"	3'–0"	3'–6"	4'–6"	6'–0"	7'–0"	8'–0"	9'–6"	11'–0"		
	550	1'–0"	1'–0"	1'–0"	2'–6"	4'–0"	4'–6"	6'–0"	7'–6"	8'–6"	9'–0"	11'–0"	12'–0"		
16"	250	1'–0"	1'–0"	1'–0"	1'–0"	1'–0"	1'–0"	1'–0"	1'–6"	2'–6"	3'–0"	5'–0"	6'–6"	9'–0"	11'–0"
	350	1'–0"	1'–0"	1'–0"	1'–0"	1'–0"	1'–0"	1'–6"	3'–0"	4'–6"	5'–0"	6'–6"	8'–0"	10'–6"	12'–0"
	550	1'–0"	1'–0"	1'–0"	1'–0"	1'–6"	2'–0"	3'–6"	5'–0"	6'–0"	7'–0"	8'–6"	10'–0"	12'–0"	13'–6"

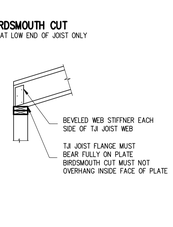
CHART B – SQUARE OR RECTANGULAR HOLES

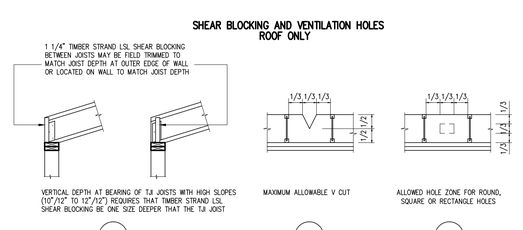
MINIMUM DISTANCE FROM INSIDE FACE OF ANY SUPPORT TO NEAREST EDGE OF HOLE

DEPTH	TJI/PRO™	ROUND HOLE SIZE												
		2"	3"	4"	5"	6"	6 1/4"	7"	8"	8 5/8"	9"	10"	10 3/4"	12"	12 3/4"
9 1/2"	150	1'–0"	2'–0"	4'–0"	6'–0"										
	250	1'–0"	2'–6"	4'–0"	6'–6"										
11 7/7"	150	1'–0"	1'–0"	1'–6"	4'–0"	7'–0"	7'–6"	8'–0"	9'–0"						
	250	1'–0"	1'–6"	3'–0"	5'–0"	8'–0"	8'–0"	8'–6"	9'–0"						
	350	1'–0"	2'–0"	4'–0"	5'–6"	8'–6"	8'–6"	9'–6"	9'–6"						
	550	1'–0"	4'–0"	5'–6"	7'–0"	9'–6"	9'–6"	10'–0"	10'–0"	10'–6"					
14"	250	1'–0"	1'–0"	1'–6"	3'–6"	5'–6"	6'–6"	8'–0"	10'–0"	10'–6"	11'–0"	11'–6"	12'–6"		
	350	1'–0"	1'–0"	2'–0"	4'–6"	6'–6"	7'–0"	9'–0"	10'–6"	11'–0"	11'–6"	12'–0"	12'–6"		
	550	1'–0"	3'–0"	5'–0"	6'–6"	8'–6"	9'–0"	10'–0"	11'–6"	12'–0"	12'–0"	12'–6"	13'–0"		
16"	250	1'–0"	1'–0"	1'–0"	1'–0"	4'–0"	4'–6"	6'–6"	9'–0"	11'–0"	11'–6"	12'–0"	13'–0"	14'–0"	15'–0"
	350	1'–0"	1'–0"	1'–0"	2'–6"	5'–0"	5'–6"	7'–0"	10'–0"	12'–0"	12'–0"	12'–6"	13'–6"	14'–6"	15–6"
	550	1'–0"	1'–6"	3'–0"	5'–0"	7'–0"	7'–6"	9'–6"	11'–6"	13'–0"	13'–6"	14'–0"	14'–6"	15'–0"	16'–0"

RECTANGULAR HOLES BASED ON MEASUREMENT OF LONGEST SIDE







ROOF DETAILS

ROOF NAILING REQUIREMENTS

- TJI® joists at end bearings: Two 10d (3") box or 12d (3 1/4") box nails (1 each side), 1 1/2" minimum from end.
- TJI® joists at intermediate bearings:

Roof slopes less than 4" per foot: Two 10d (3") box or 12d (3 1/4") box nails (1 each side). See detail R7.

Roof slopes from 4" to 5" per foot: Four 10d (3") box or 12d (3 1/4") box nails (2 each side).

Roof slopes greater than 5" per foot: Four 10d (3") box or 12d (3 1/4") box nails (2 each side) plus a twist strap and backer block.

- Blocking panels or shear blocking to bearing plate:

TJI® joist blocking panels: 10d (3") box nails at 6" on-center.

Trus Joist rim board for shear blocking:Toenail with 10d (3") box nails at 6" on-center or 16d (3 1/2") box nails at 12" on-center.

Shear transfer: Connections equivalent to decking nail schedule.

NO	REVISION DONE	DATE	BY
1	FOR PERMIT	AUGUST DATE, 2020	G.SH.

GESCON – SHAFFIR
STRUCTURAL ENGINEERS INC.
TEL (416) 636-0700
FAX (416) 636-0469
e-mail: gescon@ica.net

CLIENT

MICHAEL MANTZORIS ARCHITECT

PROJECT

510 TAUNTON ROAD EAST
OSHAWA ONTARIO

SHEET TITLE

TYPICAL DETAILS, NOTES AND TABLES

LICENSED PROFESSIONAL ENGINEER
AUGUST 10, 2020
Gideon A. Shaffir
PROVINCE OF ONTARIO

DESIGNED	G.SH
DRAWN	X.T.
CHECKED	G.SH
SCALE	3/4" : 1'-0"
DATE	AUGUST 10, 2020
PROJECT NO.	0000
DWG NO.	S-6