GENERAL NOTES

- 1. ALL DIMENSIONS AND ELEVATIONS ARE IN MILLIMETRES UNLESS NOTED OTHERWISE.
- 2. SITE VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES TO THE
- 3. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE LATEST EDITION OF ALL RELEVANT 8. CODES AND STANDARDS.
- 4. CONFORM TO OWNER'S GENERAL SPECIFICATIONS INCLUDING ALL SAFETY REQUIREMENTS.
- 5. KEEP THE SITE THROUGHOUT THE WORK AREA IN A CLEAN AND ORDERLY CONDITION AT ALL TIMES TO THE SATISFACTION OF THE OWNER.
- 6. ALL STRUCTURAL DRAWINGS ARE TO BE READ IN CONJUNCTION WITH OTHER CONSULTANTS

FOUNDATIONS

1. DESIGN BEARING PRESSURE IS ASSUMED AS 100 kPa (SLS) BASED ON THE PROPOSED GROUND IMPROVEMENT PLAN.

- 2. ALL FOOTINGS SHALL BEAR ON IMPROVED SOIL SOIL AS APPROVED BY THE GEOTECHNICAL CONSULTANT PRIOR TO PLACING FOOTING CONCRETE. REPORT ANY DOUBTFUL BEARING CONDITIONS TO THE STRUCTURAL ENGINEER BEFORE PLACING FOOTINGS.
- MATERIALS FOR BACKFILL SHALL BE GRANULAR 'A' AND GRANULAR 'B' CONFORMING TO OPSS

STANDARDS COMPACTED TO 100% STANDARD PROCTOR MAXIMUM DRY DENSITY.

- 4. ALL EXTERIOR FOOTINGS SHALL BE MINIMUM 1300mm BELOW FINISHED GRADE, UNLESS OTHERWISE NOTED.
- 5. REFER TO THE GEOTECHNICAL INVESTIGATION RPORT NUMBER 15382-00LR2 BY CAMBIUM INC. 3 DATED JANUARY 26, 2024 FOR ALL OTHER CONSIDERATIONS/ RECOMMENDATIONS WITH RESPECT TO FOUNDATION/ UNDERGROUND WORK.

CONCRETE

1. CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF CAN/CSA-A23.1 AND CAN/CSA-A23.3 WITH THE FOLLOWING PROVISIONS:

orwy ook 7/20.0 with the roccommo rikotholdid.							
LOCATION	DESIGN STRENGTH (28 DAYS)	SLUMP	EXPOSURE CLASS				
INTERIOR FOOTINGS/ FOUNDATION WALLS AND PIERS	25 MPa	80± 30	N				
EXTERIOR FOOTINGS/ FOUNDATION WALLS AND PIERS	25 MPa	80± 30	F-2				
ALL EXTERIOR REINFORCED CONCRETE (STAIRS & RETAINING WALLS)	35 MPa U.N.O.	80± 30	C-1				
EXTERIOR UN-REINFORCED CONCRETE (CURBS & CONCRETE WALKWAYS)	32 MPa U.N.O.	80± 30	C-2				
SLAB ON GRADE	25 MPa	80± 30	N				
LEAN CONCRETE	0.9 MPa	140± 30	F-2				

- 2. NO ADDITIONAL WATER SHALL BE ADDED AT THE JOB SITE. CONCRETE WHICH HAS BEEN WATERED OR DOES NOT MEET SPECIFICATIONS SHALL BE REJECTED.
- WHEN THE OUTSIDE TEMPERATURE FALLS BELOW 5°C, PROVIDE TEMPORARY HEATING OF CONCRETE IN ACCORDANCE WITH THE REQUIREMENTS OF CSA A23.1.
- 4. STRUCTURAL GROUT SHALL BE NON-SHRINK, NON METALLIC M-BED STANDARD PREMIX BY SIKA OR APPROVED EQUIVALENT.
- 5. ALL EPOXY SHALL BE HILTI HIT-HY 200 U.N.O.

SLAB ON GRADE

- 1. CAST SLAB ON GRADE ON 200mm (8")MIN CRUSHED STONE AND COMPACTED SUB-GRADE IN ACCORDANCE WITH THE GEOTECHNICAL REPORT UNLESS NOTED OTHERWISE.
- 2. REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL REQUIREMENTS OF COMPOSITION OF MATERIALS BELOW GRADE (INSULATION AND VAPOUR BARRIER).
- 3. SAWCUT WITHIN 6 TO 18 HOURS. REFER TO THE DRAWINGS FOR SAWCUT REQUIREMENTS.
- 4. DO NOT CAST SLAB MORE THAN 30 METERS IN LENGTH IN EITHER DIRECTION. PLACE SLAB IN STRIP PATTERN. KEY CONSTRUCTION JOINTS AS DETAILED.
- 5. MAINTAIN MINIMUM SPECIFIED THICKNESS AT ALL DEPRESSIONS AND CHANGES IN ELEVATIONS.
- 6. REFER TO ARCHITECTURAL DRAWINGS FOR EXTENT AND LOCATION OF ALL FINISHES, DEPRESSIONS AND SLOPES.
- 7. WELDED WIRE MESH REINFORCING IN SLABS ON GRADE MUST BE PROPERLY CHAIRED. LIFTING 1. OF THE WIRE MESH DURING POURS WILL NOT BE ACCEPTED.

CONCRETE REINFORCEMENT

1. THE CLEAR DISTANCE BETWEEN REINFORCING STEEL AND SURFACE OF CONCRETE SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:

TOLLOWS UNLESS NOTED OTHERWISE.						
LOCATION	CLEAR COVER					
FOOTINGS	75mm UNDERSIDE 50mm TOP AND ENDS					
WALLS	50mm AGAINST EARTH (20M BAR OR GREATER) 40mm AGAINST EARTH (15M BAR) 40mm AGAINST FORM (20M BAR OR GREATER) 25mm AGAINST FORM (15M BAR)					
SLABS	25mm TOP BARS 25mm BOTTOM BARS					
COLUMNS/PIERS	40mm TO TIES					
SURFACE IN CONTACT WITH GROUND	75mm					

- 2. DETAIL REINFORCING STEEL IN ACCORDANCE WITH "REINFORCING STEEL MANUAL OF STANDARD PRACTICE" BY THE REINFORCING STEEL INSTITUTE OF CANADA LATEST EDITION.
- 3. REINFORCING BAR SPLICES FOR DEFORMED BARS: WALLS - CLASS 'B' TENSION SPLICE UNLESS NOTED OTHERWISE
- ALL OTHERS CLASS 'B' TENSION LAP UNLESS NOTED OTHERWISE
- 4. ALL REINFORCING STEEL SHALL BE DEFORMED HARD GRADE BILLET STEEL CONFORMING TO CSA G30.18 GRADE 400.
- WELDED STEEL WIRE FABRIC, PLAIN TYPE CONFORMING TO CSA G30.5M IN FLAT SHEETS NOT
- 6. ALL CONCRETE REINFORCEMENT MUST BE PROPERLY CHAIRED WITH APPROVED BAR SUPPORTS.
- 7. PROVIDE CHAIRS, SPACER BARS, SUPPORT BARS AND OTHER ACCESSORIES TO SUPPORT REINFORCING IN ACCORDANCE WITH THE LATEST EDITIONS OF CSA A23.1 AND CSA

8. CHAIRS SHALL BE SPACED AT 1200mm O.C. MAXIMUM. LIFTING IS NOT ACCEPTABLE.

STRUCTURAL STEEL 1. STRUCTURAL STEEL HSS AND W SECTIONS SHALL BE G40.21M-350W CLASS C. ALL OTHERS SHALL BE G40.21M-300W

- 2. DESIGN FORCES INDICATED ON DRAWINGS FOR STRUCTURAL STEEL WORK ARE UN-FACTORED FORCES UNLESS NOTED OTHERWISE.
- 3. PREPARE AND SUBMIT SHOP DRAWINGS OF COMPONENTS AND CONNECTIONS. ALL CONNECTIONS MUST BE DESIGNED BY THE FABRICATOR'S ENGINEER AND SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THAT ENGINEER.
- 4. FABRICATORS ENGINEER MUST BE PRACTICING PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF ONTARIO.
- 5. BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO BOLTS IN EACH CONNECTED PIECE.

6. FABRICATION, ERECTION AND WORKMANSHIP SHALL CONFORM TO CSA S16.1, LATEST EDITION. ALL WELDING SHALL CONFORM TO CSA W59 AND SHALL BE PERFORMED BY A WELDER QUALIFIED UNDER CSA W47.

- ALL CONNECTIONS SHALL BE WELDED USING E49XX ELECTRODES OR BOLTED USING ASTM A325 HIGH STRENGTH BOLTS.
- ALL STRUCTURAL STEEL SHALL BE PAINTED WITH ONE SHOP APPLIED COAT OF PRIMER. SPOT PRIME ALL WELDED AREAS. SPOT PRIME AS REQUIRED.
- 10. REMOVE PAINT FILM FROM ALL STEEL SURFACES TO BE WELDED.
- DO NOT CUT OR CORE ANY OPENINGS IN ANY STRUCTURAL STEEL MEMBERS WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER
- 12. WHERE A STRUCTURAL STEEL SHAPE SHOWN ON THE DRAWINGS IS UNAVAILABLE, A SHAPE OF EQUAL OR GREATER SECTION PROPERTIES AND STRUCTURAL CAPACITY SHALL BE SUBSTITUTED UPON APPROVAL BY OWNER AND CONSULTANT AT NO EXTRA COST.

- WOOD CONSTRUCTION SHALL CONFORM TO CSA STANDARD 086 AND TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE.
- LUMBER: UNLESS OTHERWISE NOTED. TO BE SPRUCE, GRADE NO.2 CONFORMING TO CSA. STANDARD 0141 WITH MAXIMUM MOISTURE CONTENT OF 15% AT THE TIME OF INSTALLATION. LUMBER SHALL BEAR THE GRADING STAMP OF AN AGENCY APPROVED BY THE CANADIAN LUMBER B STANDARDS ADMINISTRATION BOARD.
- NAILS, SPIKES AND STAPLES. TO CSA STANDARD B111, GALVANIZED FOR EXTERIOR WORK, OR HIGHLY HUMID AREAS AND FOR TREATED LUMBER, PLAIN ELSEWHERE. NAILING OF FRAMING UNLESS OTHERWISE NOTED, SHALL CONFORM TO TABLES 9.23.3 A&B IN THE ONTARIO BUILDING
- FASTENINGS FOR EXTERIOR WORK: NAILS, BOLTS, STEEL STRAPS AND WELDED CONNECTIONS TO
- 5. LUMBER TO LUMBER CONNECTIONS IN SAME PLANE SHALL BE MADE WITH APPROVED JOIST
- HANGERS OR FRAMING ANCHORS. 6. WOOD PRESERVATIVE: WHERE REQUIRED. TO CONFORM TO CSA STANDARD 080.

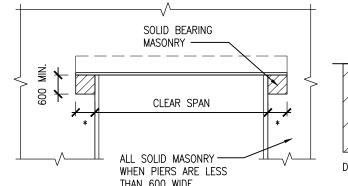
BE HOT DIP GALVANIZED AND CONFORM TO CAN3-86-M80.

- 7. FRAMING ANCHORS: FRAMING ANCHORS, JOIST HANGERS, UNLESS OTHERWISE SHOWN ON THE DRAWINGS ARE ALL TO BE AS MANUFACTURED BY SIMPSON STRONG TIE OR AN APPROVED. EQUAL, SIZED TO THE JOB AT HAND. ALL ARE TO BE INSTALLED IN STRICT ACCORDANCE WITH NAILS WHERE "SPECIAL" S INSTRUCTIONS UTILIZING THE MANUFACTURER REQUIRED.
- 8. REMOVE AND REPLACE ANY DEFECTIVE MATERIALS WHEREVER FOUND PRIOR TO FINAL ACCEPTANCE OF THE WORK.
- 9. ALL TIMBER CONNECTIONS SHALL BE IN ACCORDANCE WITH THE REFERENCE STANDARD AND WITH 10. DESIGN OF TRUSSES SHALL CONFORM TO PART 4 OF OBC ON SHOP DRAWINGS SEALED BY A
- PROFESSIONAL ENGINEER REGISTERED IN ONTARIO. 11. UNLESS OTHERWISE NOTED ON PLAN, THE FOLLOWING SHALL BE THE MINIMUM GRADES USED:
- 12. PROVIDE ERECTION DRAWINGS IN ACCORDANCE WITH M.O.L. STANDARDS FOR SEQUENTIAL
- 13. DO NOT NOTCH OR DRILL LUMBER TRUSSES ON SITE WITHOUT MANUFACTURER'S APPROVAL. REFER TO MANUFACTURER'S PRINTED INFORMATION FOR OPENINGS IN LUMBER FRAME MEMBERS.
- 14. PROVIDE LATERAL RESTRAINT AT ALL BEARING LOCATIONS AND ONE ROW OF BRIDGING AT ALL MIDSPANS UNLESS NOTED OTHERWISE. DRAWINGS SHOWING CONNECTION DETAILS, CONSTRUCTION DETAILS, AND TEMPORARY CONSTRUCTION BRACING, ALL SHOP DRAWINGS TO BE PREPARED AND FTG APPROVED BY A REGISTERED PROFESSIONAL ENGINEER IN THE PROVINCE OF ONTARIO.
- 15. SUBMIT SHOP DRAWINGS OF PREFAB ENGINEERED LUMBER ROOF TRUSS FRAMING AND ERECTION. HB
- 16. INSTALL ALL LUMBER TRUSSES TO MANUFACTURER'S RECOMMENDED DETAILS INCLUDING ALL NECESSARY BLOCKING, WEB STIFFENERS AND BRACING.
- 17. UPON INSTALLATION OF THE WOOD TRUSSES THE MANUFACTURER'S SPECIALITY ENGINEER SHALL HK SUBMIT A LETTER OF FIELD REVIEW AND COMPLIANCE CONFIRMING THAT THE FABRICATION AND H/HORI7 INSTALLATION OF THE TRUSS ARE IN CONFORMITY WITH THE SHOP DRAWINGS.
- 18. REFER TO THE ROOF PLAN FOR THE TRUSS DESIGN LOADS. THE TRUSSES SHOULD ALSO BE DESIGNED FOR A VERTICAL POINT LOAD OF 0.1 KN (UNFACTORED) APPLIED AT ANY POINT OF THE BOTTOM CHORD.
- 19. THE WOOD TRUSSES ARE TO BE DESIGNED FOR A LIVE LOAD DEFLECTION OF 1/360 OF THE SPAN. WOOD TRUSS DESIGN TO LATERALLY BRACE SUPPORTING BEAM AND COLUMN.

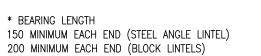
ENGINEERED WOOD TRUSSES

- ALL ENGINEERED WOOD TRUSSES TO BE DESIGNED FOR LOADING AS SHOWN ON THE
- STRUCTURAL DRAWINGS. TRUSS MANUFACTURER TO SUPPLY AND INSTALL ALL NECESSARY AND ADEQUATE HARDWARE, INCLUDING ANY JOIST HANGERS AND/ON NAILERS TO COMPLETE ALL STRUCTURAL DETAILS FOR TRANSFER OF LOADS TO STEEL BEAMS AND COLUMNS.
- PROVIDE AT LEAST 2 ROWS OF "X" BRIDGING AT 3RD POINTS OF SPAN OF JOIST FOR
- STIFFENING AND LOAD SHARING. 4. JOIST MANUFACTURER TO SUPPLY ADEQUATE BEARING STIFFENER AT EACH END OF JOIST FOR
- TRANSFER OF REACTIONS TO BEAMS OR COLUMNS.
- 5. THE ENGINEERED JOIST SUPPLIER SHALL SPECIFY ALL ERECTION REQUIREMENTS AND PROVIDE ALL NECESSARY, TEMPORARY AND PERMANENT BRACING.
- TOTAL LOAD (LIVE AND DEAD) DEFLECTION: FLOOR JOISTS = L/240LIVE LOAD FLOOR JOISTS = L/360
- COORDINATE WITH ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
- 8. PROVIDE MAXIMUM JOIST SIZE AND SPACING AS SHOWN ON DRAWINGS.
- 9. COMPLY WITH ALL MANUFACTURERS PUBLISHED INSTRUCTIONS/ REQUIREMENTS FOR INSTALLATION OF TJI JOISTS.
- 10. PROVIDE ENGINEERED HANGERS TO SUPPORT APPLIED LOADING AS REQUIRED.

TYPICAL DETAIL FOR LINTEL BEARING







ABBREVIATIONS

STEEL MOMENT CONNECTION \bigcirc DISTANCE TO TOP OF BEAM OR FOOTING FROM DATUM ELEVATION OF FLOOR OR DISTANCE TO TOP OF SLAB FROM FLOOR ELEVATION DENOTES COMPOSITE STEEL

ARCH

DETAIL

DOWN

DITTO

DRAWING

HEAVY DUTY MASONRY

HORIZONTAL SLOTTED

HOOKED REINFORCEMENT

HOLLOW STRUCTURAL SECTION

DESIGN LOADS:

ROOF DEAD: 1.40 kPa

FLOOR DEAD: 3.25 kPa

CORRIDOR LIVE: 4.80 kPa

RESIDENTIAL LIVE: 1.90 kPa

SNOW LOAD: Ss= 1.40 kPa

WIND LOAD: $q(\frac{1}{50})$: 0.48 kPa

IMPORTANCE : NORMAL

SEISMIC DATA:

DEAD

LIVE

LOAD: PGV:

SITE CLASS: E

BASE SHEAR:

DETAIL 2

CONCRETE:

Sr= 0.40 kPa

= 4.75 kPa

= 4.80 kPa

Sa(0.2): 0.188

Sa(1.0): 0.058

Sa(10.0): 0.003

0.107

0.0071

Sa(0.5):

Sa(2.0):

Sa(5.0):

0.119

0.086

R_0: 1.3

R_D: 3.0

R_0: 1.7

N-S: 555kN

E-W: 1184kN

REINFORCEMENT

HORIZONTAI

CONNECTION

DIAMFTER

DIAGONAL

→ DIRECTION OF SLOPE SHORING LOAD IN kN ANCHOR BOLT FACTORED AXIAL LOAD (kN). INDICATES TENSION. INDICATES COMPRESSION ABOVE FINISHED FLOOR AI TERNATE ARCHITECTURAL воттом CAMBER IN 'x' mm

NOT IN CONTRACT NEAR FACE NOT TO SCALE OUTSIDE FACE COLUMN BRACE CENTRE TO CENTRE FACTORED COMPRESSION (kN) CONTROL JOINT CONNECTION CONTINUOUS CENTRELINE COMPLETE WITH (INCLUDING) COLUMN CONCRETE CONTINUOUS DIAGONAL BRACE

SIMII AR DIMENSION STANDARD DEAD LOAD (kN/m2) STIFFENER STRUCTURAL THICKNESS TIE JOIST TOP OF REINFORCEMEN^T TYPICAL

FACH FND FACH FACI ELECTRICAL ELEVATION UNDERSIDE EQUAL VERTICAL BRACE EACH SIDE VERTICAL EACH WAY FACTORED SHEAR FORCE FXISTING **FXPANSION JOINT** VERTICAL SLOTTED EXTERIOR CONNECTION FLOOR DRAIN FACTORED WEIGHT FOUNDATION WELDED WIDE FLANGE FAR FACE FINISHED WELDED WIRE MESH FLOOR FOOTING GALVANIZED HIGH BEAM HOLLOW CORE CONCRETE

OPEN WEB STEEL JOIST POINT LOAD PRECAST CONCRETE PROJECTION ROOF DRAIN REINFORCING REVISION FACTORED VERTICAL REACTION ROUGH OPENING SUPERIMPOSED DL (EXCLUDING SELF WEIGHT) (kN/m2) STANDARD DUTY MASONRY REINFORCEMENT STEP DOWN FOOTING FACTORED TENSION FORCE TOP OF STEEL UPPER LAYER UNLESS NOTED OTHERWISE

INSIDE FACE INCLUDING

STEEL ANGLE

LOWER LAYER

MAXIMUM

MECHANICAL

MINIMUM MISCELLANEOUS

MEZZANINE

LOW BEAM

LONG

INTERIOR

COLUMN SCHEDULE DATA T.O. ROOF DECK LIVE LOAD (kN/m2) T.O. FIFTH FLOOR LONG LEG HORIZONTAL LONG LEG VERTICAL MOMENT CONNECTION TO HAVE FACTORED MOMENT CAPACITY T.O. FOURTH FLOOR FACTORED MOMENT (kN-m) FACTORED TORSION (kN-m) T.O. THIRD FLOOR T.O. SECOND FLOOR U/S OF LINTEL T.O. GROUND FLOOR TOP PLATE 16x180x300 TOP PLATE ANCHORS THROUGH BOLTS THROUGH ST1 TOP

19x300x300

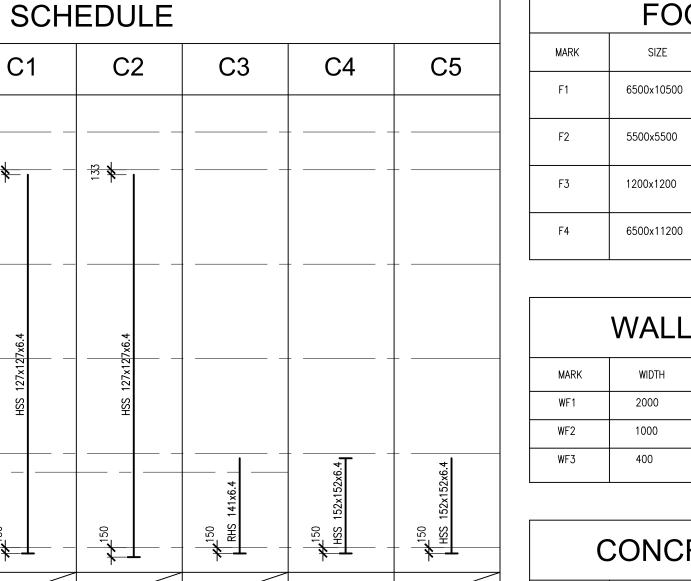
(4) 19Ø x 500

LG + 64 HOOK

19x180x350

LG + 64 HOOK

BASE ANCHOR RODS



19x180x350

ĹĠ + 64 HOOK

19x180x350

(2) 19Ø x 500

ĽG + 64 HOOK

MARK

ST1

300 C/C

LINTEL SCHEDULE						
MARK	SIZE	DETAIL	REMARKS			
L1	3 PLY 38x184		MIN. 3 PLY 38x184 AT ENDS U.N.O.			
L2	4 PLY 38x235		MIN. 4 PLY 38x184 AT ENDS U.N.O.			
L3	4 PLY 38x184		MIN. 4 PLY 38x184 AT ENDS U.N.O.			
L4	3 PLY 38x140		MIN. 3 PLY 38x140 AT ENDS U.N.O.			
L5	(3) 38x235		MIN. 3 PLY 38x184			
L6	(3) 38x235	1.90E MICROLAM LVL	MIN. 4 PLY 38x140			

19x300x300

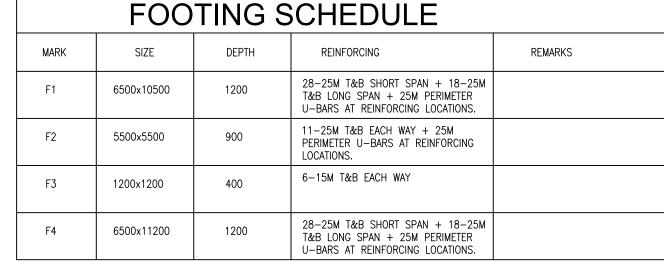
(4) 19Ø x 500

G + 64 HOOK

WALL PLATE SCHEDULE NELSON STUD DOUBLE DOUBLE TRIPLE ROW STUD ROW STUD PLATE PLATE PLATE ANCHORAGE (LENGTH) CxD mm PLATE AxBxt mm PLATE TYPE 4-19ø x 127mm LG. NELSON STUD 200x250x16 SINGLE PLATE, DOUBLE ROW 4-19ø x 127mm LG. NELSON STUD SINGLE PLATE, DOUBLE ROW WP2 400x250x16 4-19¢ NELSON STUDS 450x200x16 DOUBLE PLATE, DOUBLE ROW

SHEAR WALL SCHEDULE

SHEAR WALL ST3



	WALL FOOTING SCHEDULE							
MARK	WIDTH	DEPTH	REINFORCING	REMARKS				
WF1	2000	600	10-20M CONT. LONG 20M AT 250 TRANSVERSE					
WF2	1000	400	6-20M CONT. LONG 20M AT 350 TRANSVERSE					
WF3	400	250	3-15M CONT.					

CONCRETE WALL SCHEDULE							
MARK	THICKNESS	VERT. REINFORCING	HORIZ. REINFORCING	REMARKS			
CW1	250	15M AT 400 C/C	15M AT 400 C/C				
CW2	250	SEE SHEAR WALL SCH	EDULE				
CW3	250	15M AT 400 c/c E.F.	15M AT 400 c/c E.F.				
CW4	200	15M AT 400 c/c	15M AT 400 c/c				
CW5	455	15M AT 400 c/c E.F.	15M AT 400 c/c E.F.				
CW6	200	15M AT 400 c/c E.F.	15M AT 400 c/c E.F.				
	1	<u>'</u>					

S			UT THE PERMISSION OF THE CONSULTANT IS		
			NGS ARE NOT TO BE USED FOR CONSTRUCT EALED BY THE CONSULTANT.	ION UNTIL S	SIGNED
		NO.	ISSUES	DATE	BY
		1	ISSUED FOR PERMIT	APRIL 30, 2024	BBA
		2	RE-ISSUED FOR PERMIT	JULY 23, 2024	BBA
	1				1

1	ISSUED FOR PERMIT	APRIL 30, 2024	BBA
2	RE-ISSUED FOR PERMIT	JULY 23, 2024	BBA

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	200	15M AT 400 c/c	15M AT 400 c/c				
	455	15M AT 400 c/c E.F.	15M AT 400 c/c E.	F.			
	200	15M AT 400 c/c E.F.	15M AT 400 c/c E.F.				
					1		
ς.	THD /	NALL SCH					
J	י טט י						
9	SIZE/ SPACING	SILL PLATE	TOP PLATE	SHEATHING	1		
	SIZE, SI AOIIVO	SILL I DAIL	TOT TEXTE	SILATINO			
		38x184 C/W 16Ø x 200 EMI EPOXY ANCHORS AT 800 C/O		13 EXT. PLYWOOD SHEATHING C/W STAGGERED JOINTS			

13 PLYWOOD SHEATHING C/W

STAGGERED JOINTS (BOTH SIDES)

CONCRETE PIER SCHEDULE

| 38×140 @ | 38×140 C/W 16ø x 500 LG+ 50 | 2-38×140

HOOK ANCHORS AT 800 C/C

ALL PIERS TO BE CENTRED ON COLUMNS UNLESS NOTED OTHERWISE.

C/W 16ø x 508mm LG. +

50mm HOOK ANCHORS AT

800mm C/C. 2-38x184 WOOD TOP PLATE.

. CARRY ALL HORIZONTAL WALL REINFORCMENT THROUGH PIERS.

SEE SHEAR WALL SCHEDULE

MARK	SIZE	VERT. REINF.	TIES	REMARKS	DETAIL
P1	400x400	8-15M VERT.	10M AT 300	DETAIL 1	DETAIL 1

PROJECT:

NO.

REVISIONS

MIXED-USE BUILDING **DEVELOPMENT (BLDG.#3)** PHASE 1 1697 HIGHWAY#2

DATE

R.H. Gay Holdings Co.

COURTICE, ON

DRAWING:

GENERAL NOTES & SCHEDULES



BARRY BRYAN **ASSOCIATES** Architects Engineers Project Managers

250 Water Street Suite 201 Whitby, Ontario L1N 0G5 Tel: (905) 666-5252 Fax: (905) 666-5256

21046

PROJECT NO:

e-mail: bba@bba-archeng.com FILE:

> DRAWING NO: **S101**

DESIGN BY:

DRAWN BY:

CHECKED BY:

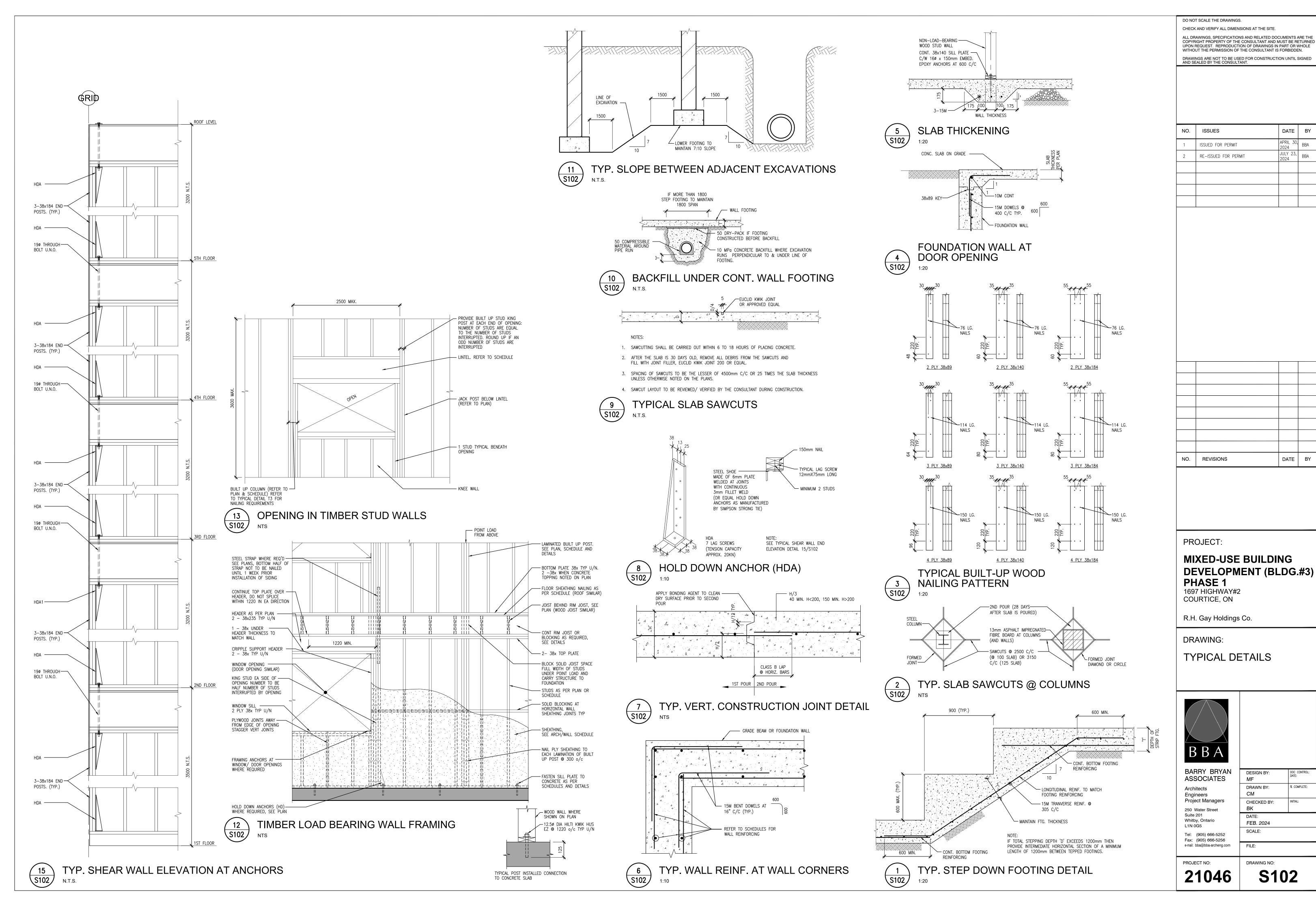
FEB. 2024

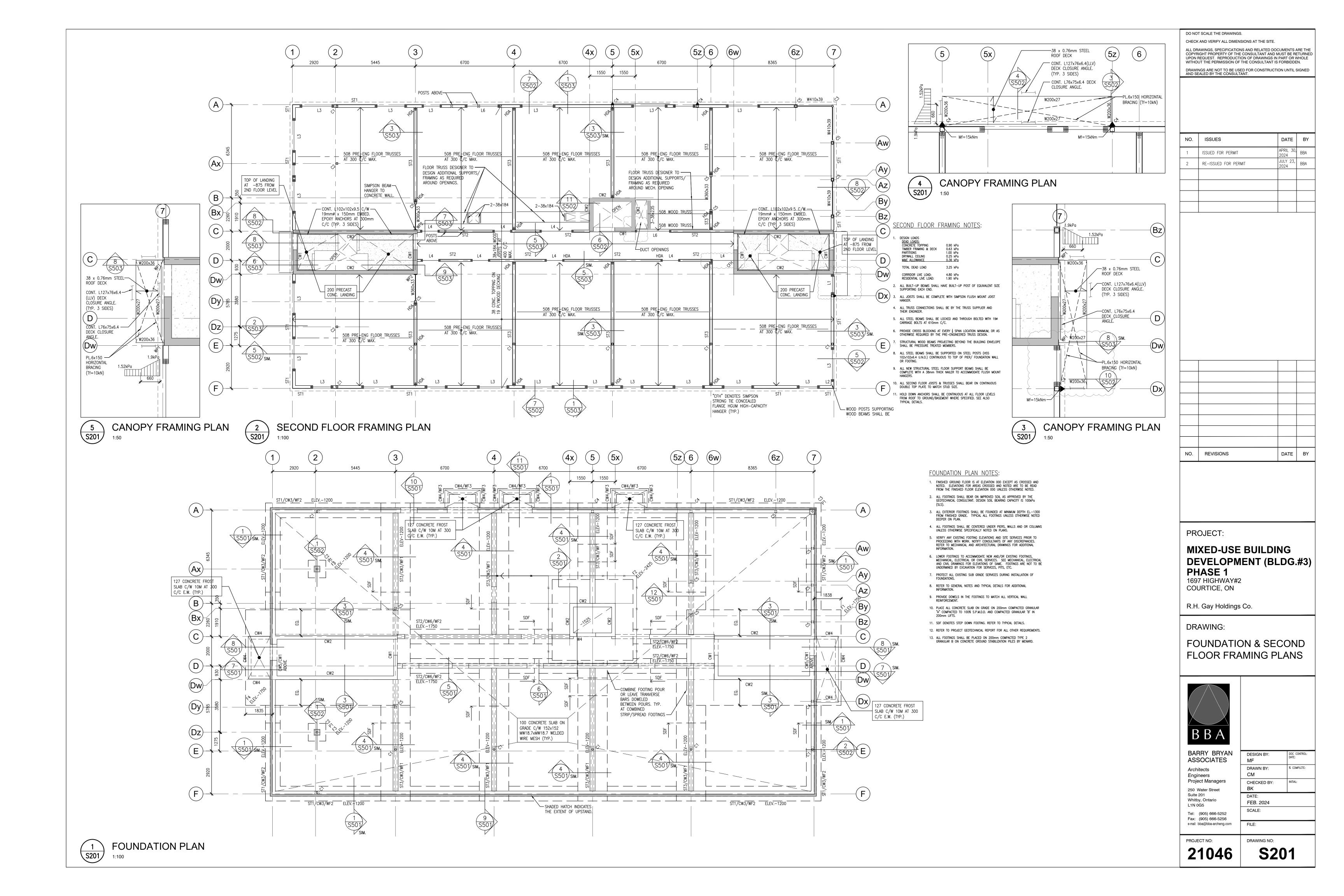
BK

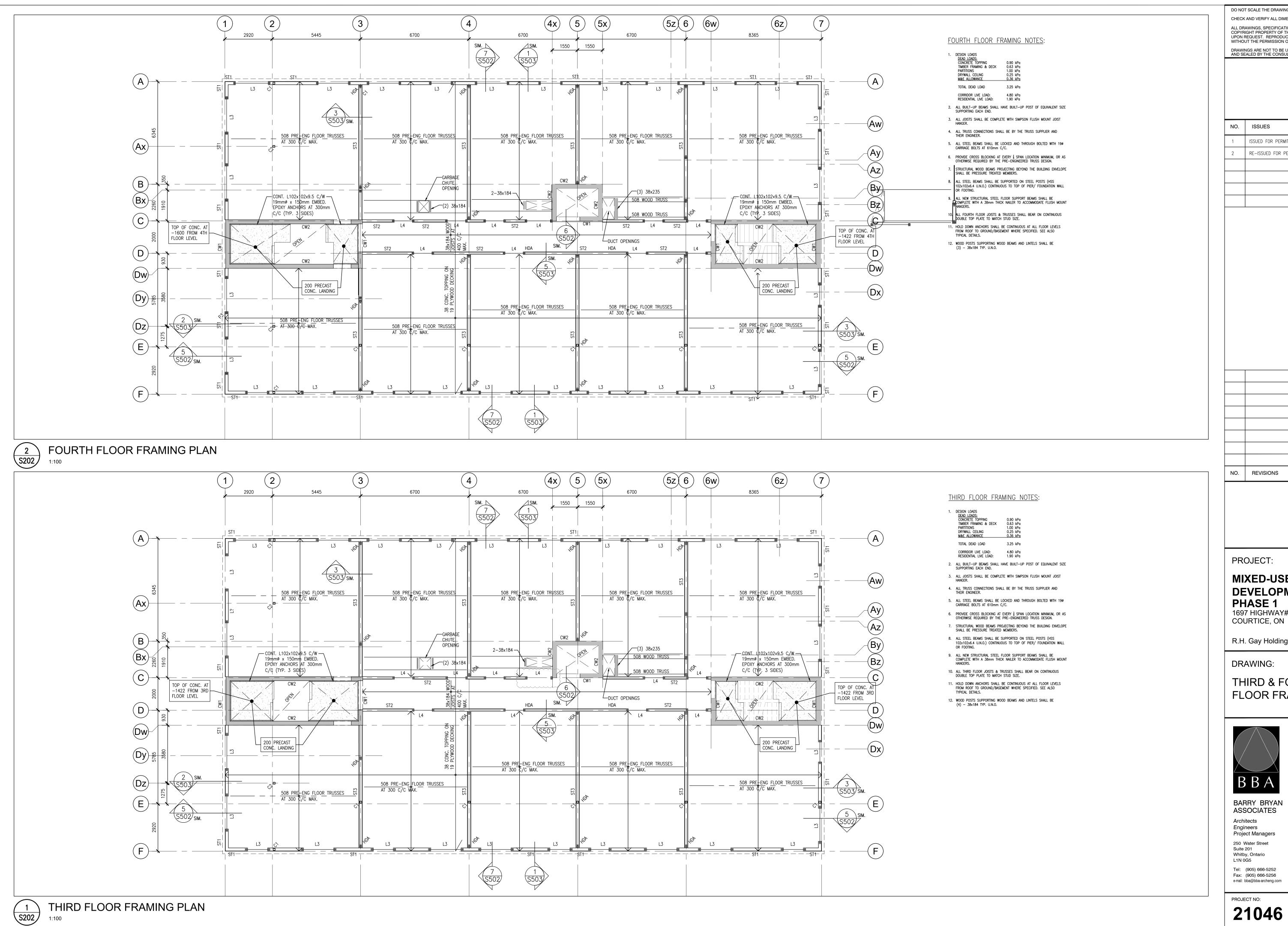
DOC CONTROL:

% COMPLETE:

REFER TO PLANS — 15M AT 400 C/C V.E.F. + — (4)−15M V.E.F. CONCENTRATED BARS AT ENDS, TYP. + 10M 10M TIES AT 300 C/C H.E.F. TIES AT 300 C/C SHEAR WALL CW2 REFER TO PLANS 13mm SPF PLYWOOD E.F. c/w 38x184 WOOD BLOCKING - 38x184 WOOD STUDS — SEE 15/S102 FOR REFER TO 15/S102 — AT 1000mm C/C MAX. AT 305mm C/C 64mm LG. x 3.33mm DIA. HOLD DOWN ANCHORS. FOR CHORD MEMBERS NAILS AT 300mm C/C AND AT 50mm C/C AT PANEL ENDS. ─ 38x184 WOOD SILL PLATE







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NO. ISSUES DATE BY ISSUED FOR PERMIT RE-ISSUED FOR PERMIT BBA

NO. REVISIONS DATE BY

PROJECT:

MIXED-USE BUILDING DEVELOPMENT (BLDG.#3) PHASE 1

1697 HIGHWAY#2 COURTICE, ON

R.H. Gay Holdings Co.

DRAWING:

THIRD & FOURTH FLOOR FRAMING PLANS



BARRY BRYAN ASSOCIATES Architects Engineers Project Managers 250 Water Street

DRAWN BY: CHECKED BY: BK FEB. 2024

FILE:

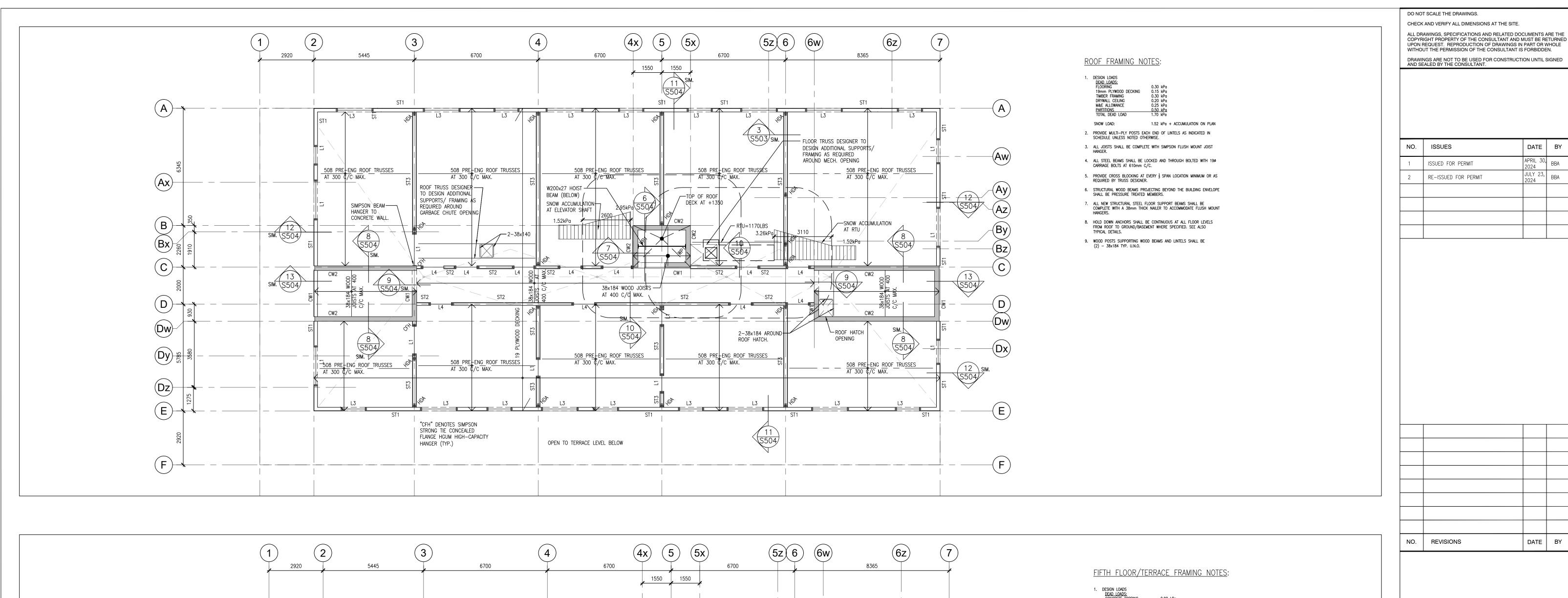
DESIGN BY:

Whitby, Ontario L1N 0G5 Tel: (905) 666-5252 Fax: (905) 666-5256 e-mail: bba@bba-archeng.com

DRAWING NO:

S202

% COMPLETE:



L3

508 PRE ENG FLOOR TRUSSES
AT 300 C/C MAX.

508 WOOD TRUSS

—(3) 38x235

508_PRE_ENG_FLOOR_TRUSSES AT 300 C/C MAX.

W410x54 + 2-38x184 CONT.

ST1

ST2

DUCT OPENING

HDA

DESIGN ADDITIONAL SUPPORTS/

FRAMING AS REQUIRED AROUND MECH. OPENING

L3

SIM.

ST2

2-38x184-

L4 HDA

508 PRE ENG FLOOR TRUSSES
AT 300 C/C MAX.

W410x54 + 2-38x184 CONT.

ST2 8 SIO O X

—HATCHED AREA DENOTES

EXTENT OF TERRACE ABOVE

508 PRE ENG FLOOR TRUSS

FLOOR TRUSS DESIGNER TO _____

ST2

508 PRE ENG FLOOR TRUSSES
AT 300 C/C MAX.

W410x54 \pm 2-38x184 CONT.

DESIGN ADDITIONAL SUPPORTS/

FRAMING AS REQUIRED

OPENING

- CONT. L102x102x9.5 C/W -19mmø x 150mm EMBED. EPOXY ANCHORS AT 300mm

200 PRECAST

508 PRE ENG FLOOR TRUSSES
AT 300 C/C MAX.

W410xb4 + 2-38x184 CONT.

508 PRE ENG FLOOR TRUSSES

CONC. LANDING

(S504)

S504)

ST1 |

AROUND GARBAGE CHUTE

1. DESIGN LOADS
DEAD LOADS:
CONCRETE TOPPING
TIMBER FRAMING & DECK
PARTITIONS
DRYWALL CEILING
M&E ALLOWANCE TOTAL DEAD LOAD 3.25 kPa CORRIDOR LIVE LOAD: RESIDENTIAL LIVE LOAD: TERRACE LIVE LOAD:

ST1

508 PRE ENG FLOOR TRUSSES AT 300 C/C MAX.

CONT. L102x102x9.5_C/W — 19mmø x 150mm EMBED. EPOXY ANCHORS AT 300mm

200 PRECAST

508 PRE ENG FLOOR TRUSSES AT 300 C/C MAX.

W410x54 + 2-38x184 CONT.

CONC. LANDING

C/C (TYP. 3 SIDES)

CW2

-(Aw)

(Bz)

-(Dx)

TOP OF CONC. AT

-1422 FROM 5TH FLOOR LEVEL

2. ALL BUILT-UP BEAMS SHALL HAVE BUILT-UP POST OF EQUIVALENT SIZE SUPPORTING EACH END.

3. ALL JOISTS SHALL BE COMPLETE WITH SIMPSON FLUSH MOUNT JOIST

4. ALL TRUSS CONNECTIONS SHALL BE BY THE TRUSS SUPPLIER AND THEIR ENGINEER. ALL STEEL BEAMS SHALL BE LOCKED AND THROUGH BOLTED WITH 19Ø CARRIAGE BOLTS AT 610mm C/C.

6. PROVIDE CROSS BLOCKING AT EVERY \$\frac{1}{3}\$ SPAN LOCATION MINIMUM, OR AS OTHERWISE REQUIRED BY THE PRE-ENGINEERED TRUSS DESIGN.

7. STRUCTURAL WOOD BEAMS PROJECTING BEYOND THE BUILDING ENVELOPE SHALL BE PRESSURE TREATED MEMBERS.

8. ALL STEEL BEAMS SHALL BE SUPPORTED ON STEEL POSTS (HSS

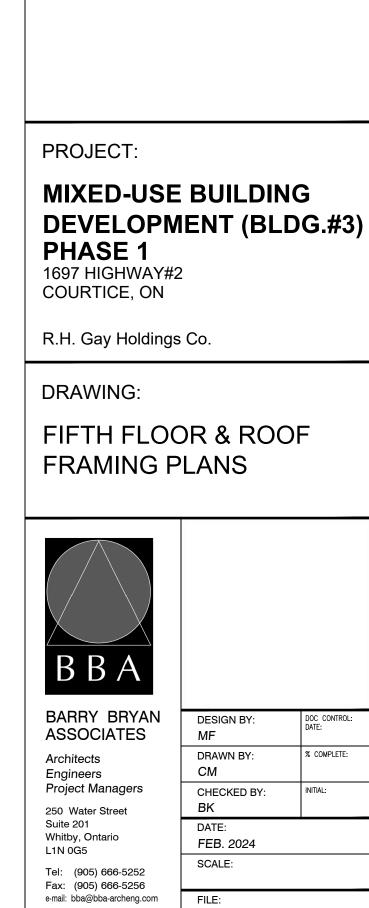
102x102x6.4 U.N.O.) CONTINUOUS TO TOP OF PIER/ FOUNDATION WALL OR FOOTING. ALL NEW STRUCTURAL STEEL FLOOR SUPPORT BEAMS SHALL BE COMPLETE WITH A 38mm THICK NAILER TO ACCOMMODATE FLUSH MOUNT HANGERS.

10. ALL FIFTH FLOOR & TERRACE JOISTS & TRUSSES SHALL BEAR ON CONTINUOUS DOUBLE TOP PLATE TO MATCH STUD SIZE.

11. HOLD DOWN ANCHORS SHALL BE CONTINUOUS AT ALL FLOOR LEVELS FROM ROOF TO GROUND/BASEMENT WHERE SPECIFIED. SEE ALSO

12. WOOD POSTS SUPPORTING WOOD BEAMS AND LINTELS SHALL BE (3) - 38x184 TYP. U.N.O.

13. NOTE: LOVE LOAD ON TERRACE ROOF SUPERCEDES SNOW DRIFTING.



FIFTH FLOOR/ TERRACE FRAMING PLAN S203 1:100

14 \$504 SIM.

TOP OF CONC AT

-1600 FROM 5TI

FLOOR LEVEL

508 PRE-ENG 'S

T508 PRE-ENG-17
FLOOR TRUSSES

AT 300 C/C

(Ax)

(B)

Bx

(D)

(Dz)-

(E)-

PROJECT NO: 21046

DRAWING NO: **S203**

% COMPLETE:

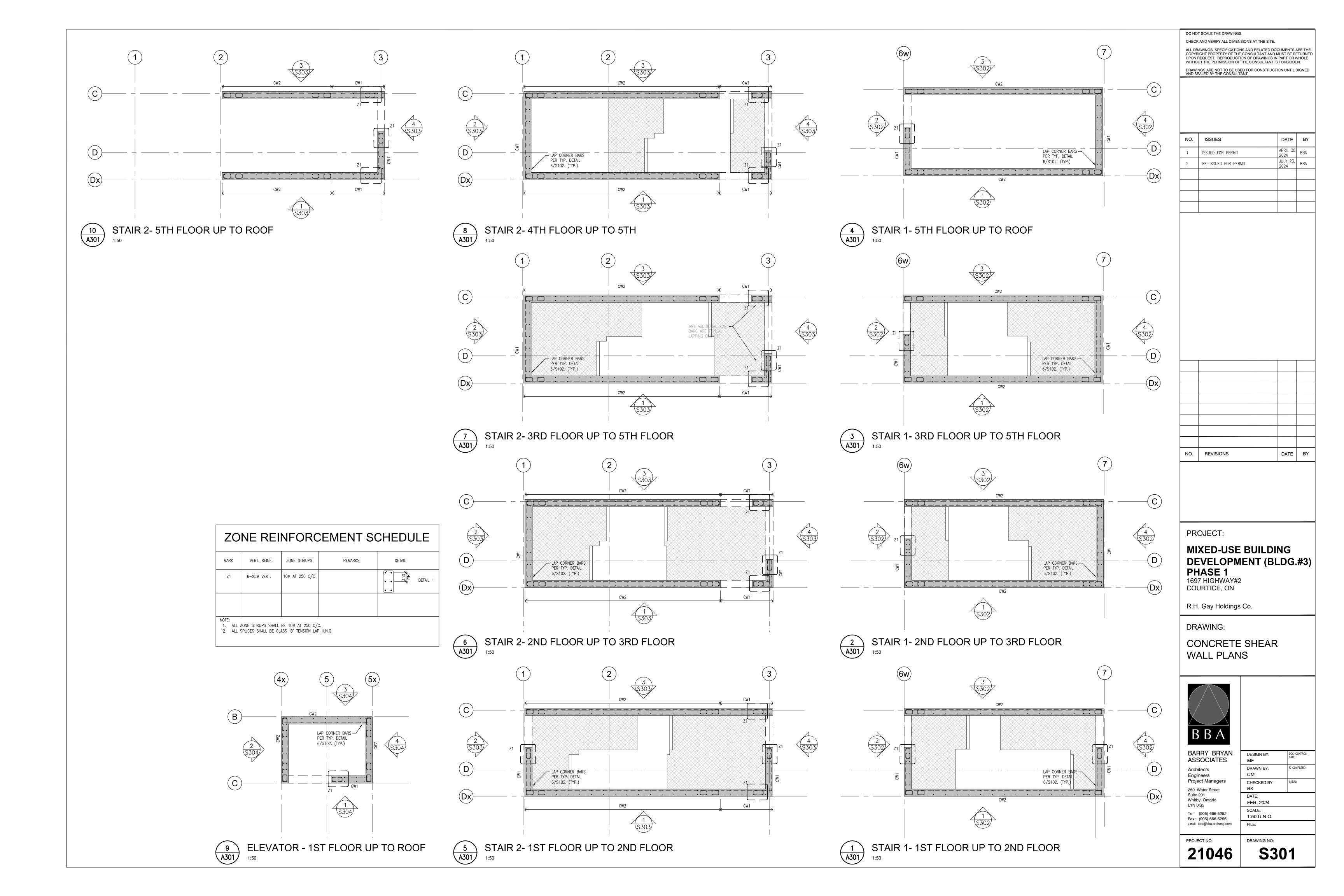
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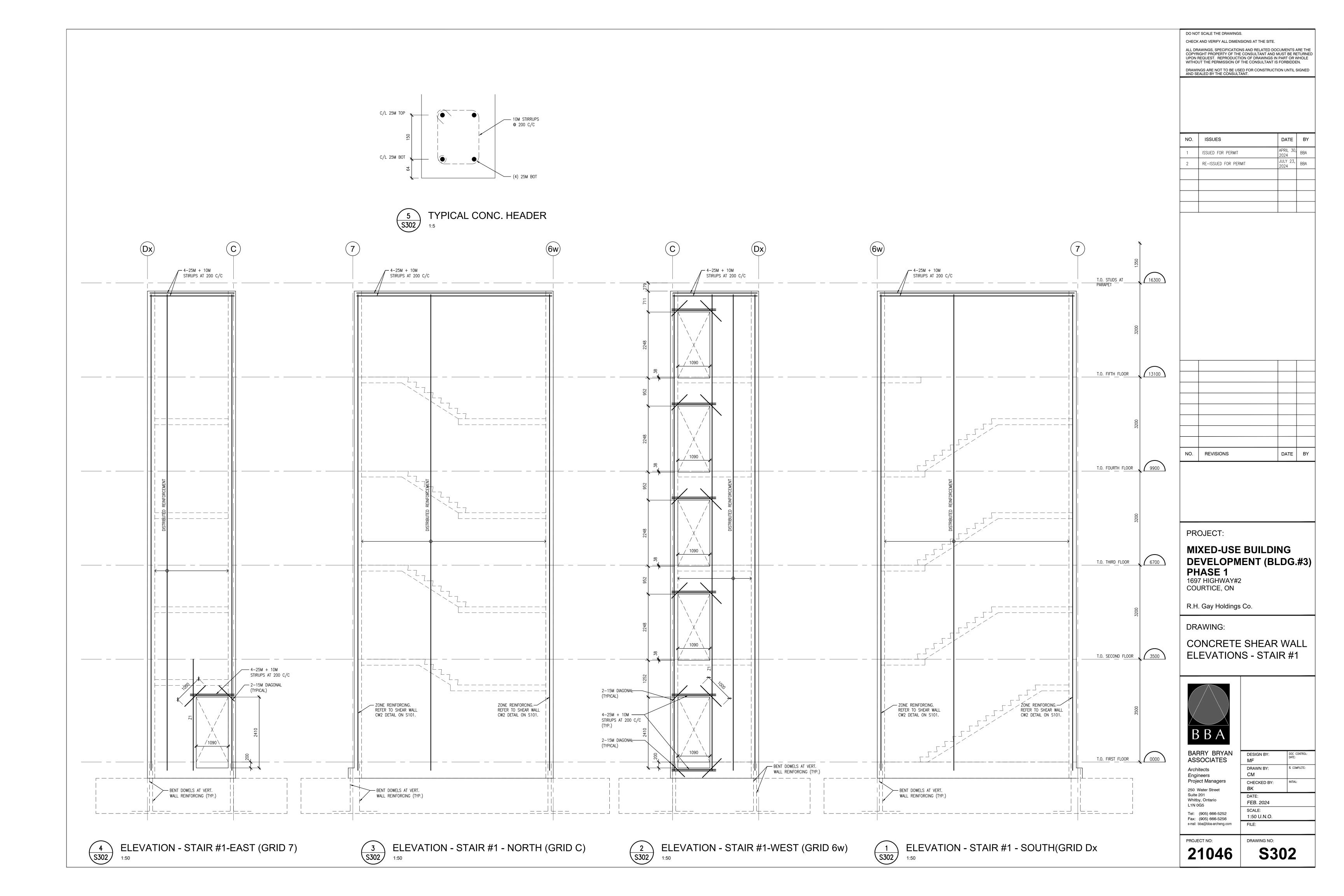
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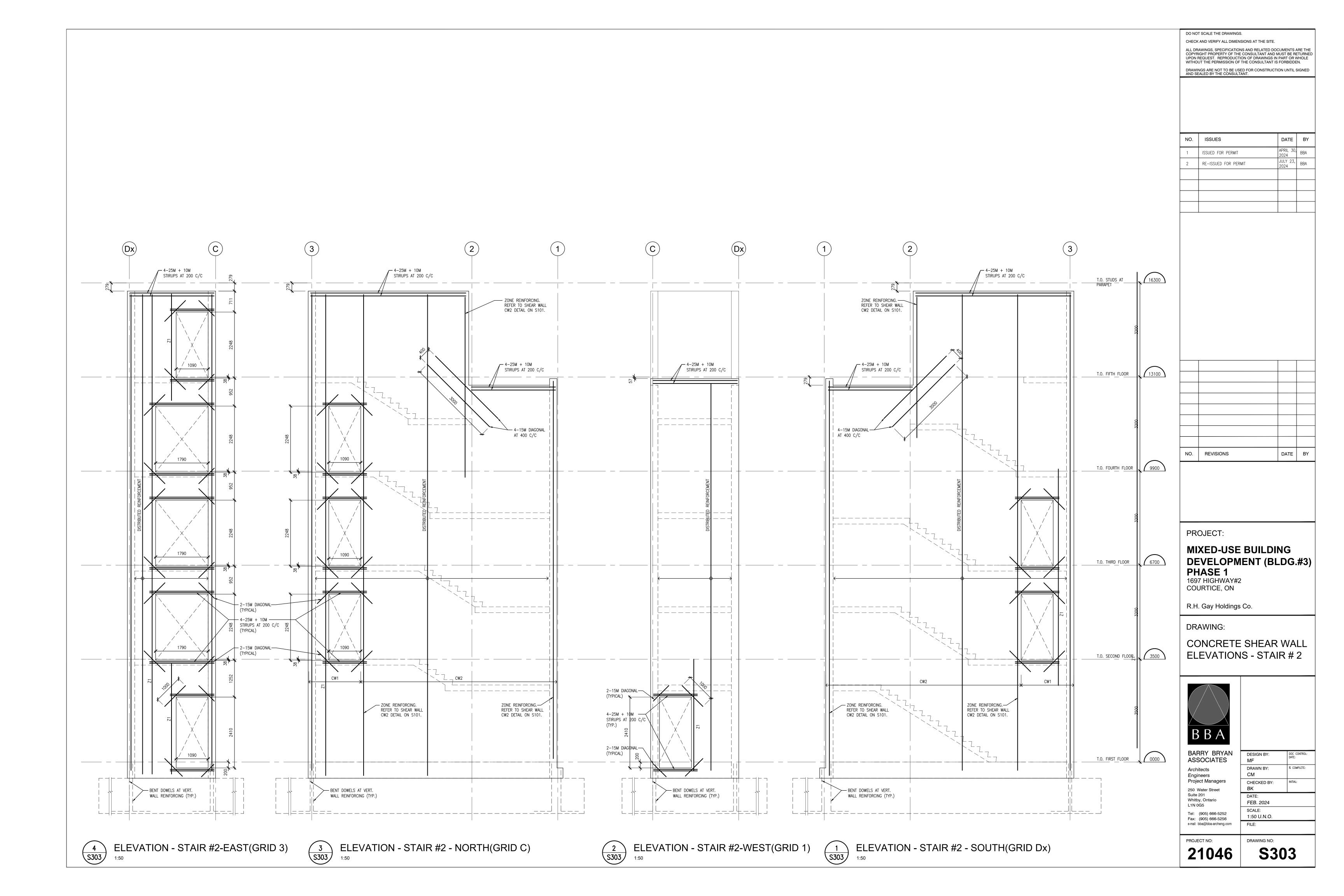
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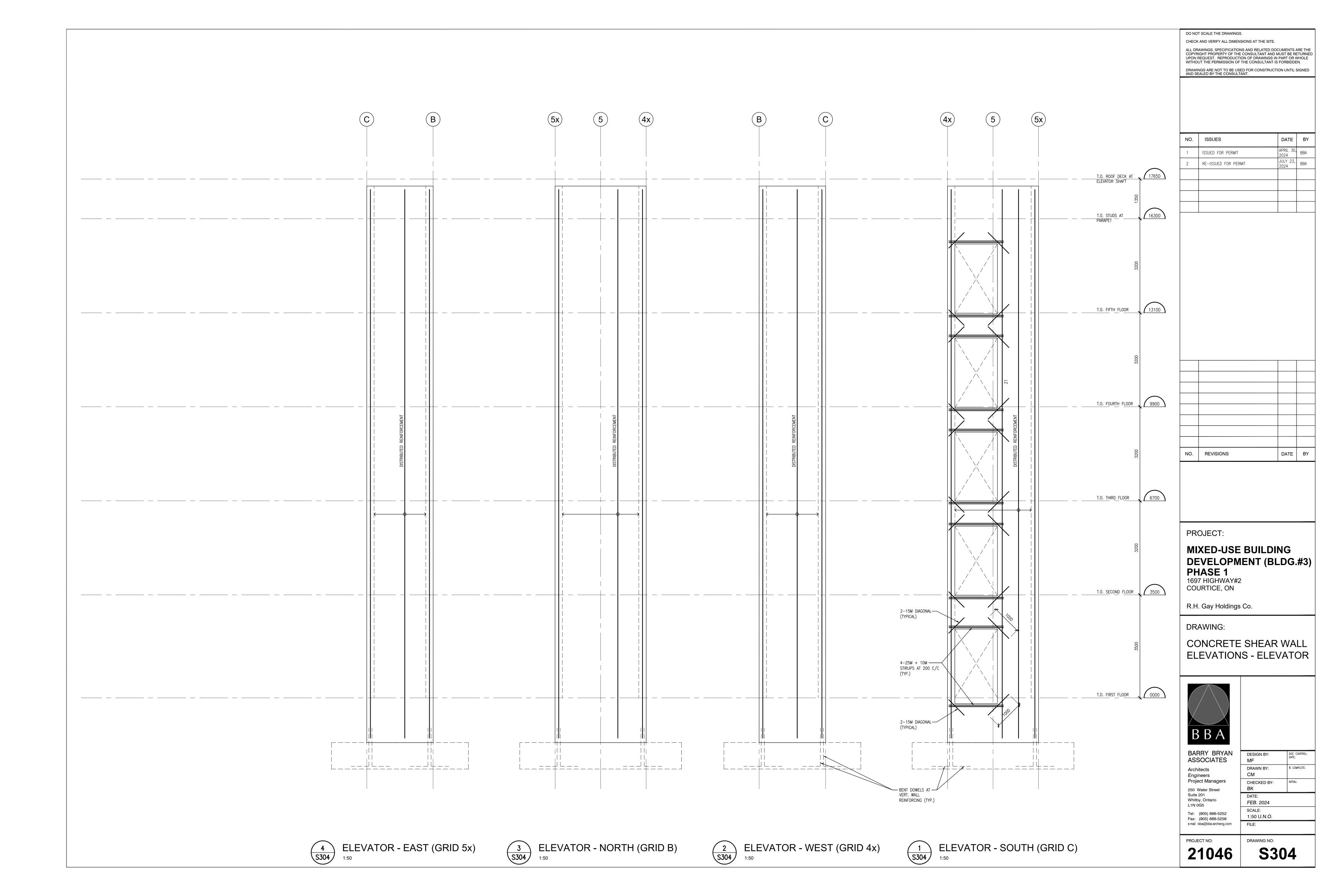
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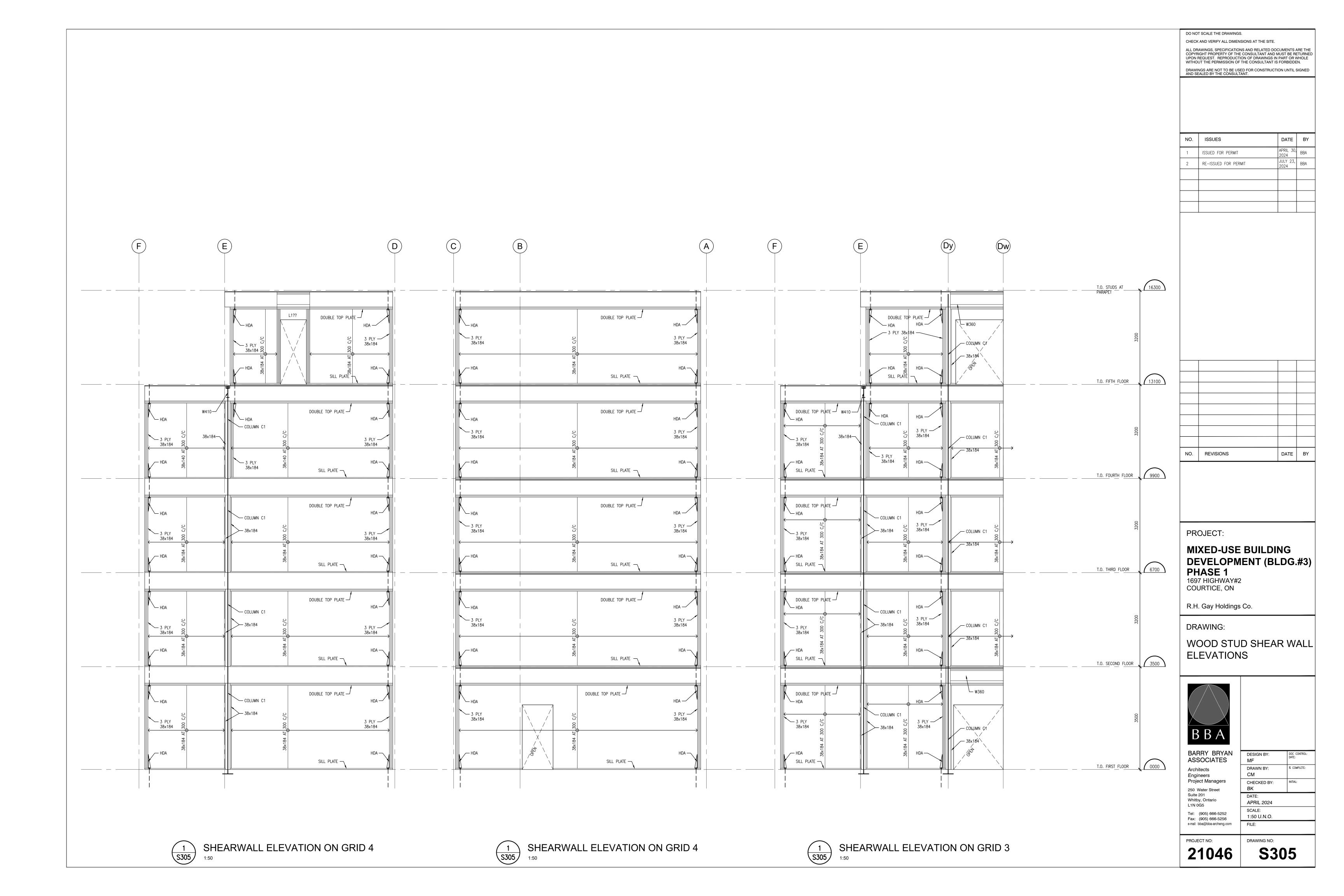
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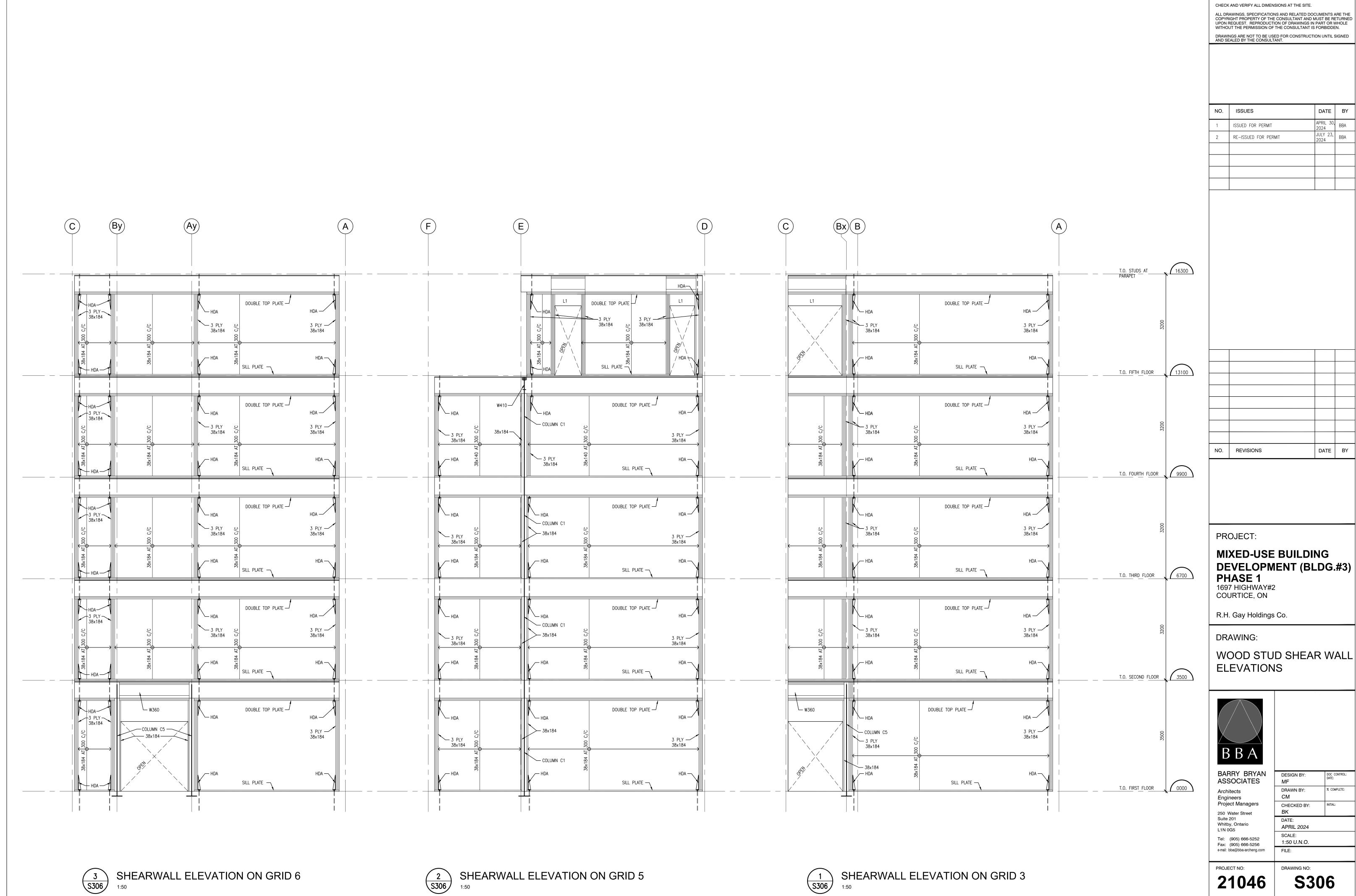












DO NOT SCALE THE DRAWINGS.

